Hillsborough County Tax Collector Southshore Office 410 30th Street SE Ruskin, Florida 33570





Project Team:

Owner:

Hillsborough County Tax Collector 601 E. Kennedy Blvd., 14th Floor Tampa, FL 33602 813.635.5200 www.hillstax.org Architect:

B Frank Studio, LLC 4836 West Gandy Boulevard Tampa, FL 33611 813.769.9378 www.bfrankstudio.com Construction Manager:

Manhattan Construction Company 1715 N. Westshore Blvd., Suite 175 Tampa, FL 33607 813.675.1960 www.manhattanconstructiongroup Civil Engineer:

Florida Engineering and Environmental Services, Inc. 4519 George Road, Suite 130 Tampa, FL 33634 813.880.9106 www.flaengineering.com Structural Engineer:

Adams Engineers and Consultants, Inc. 5507 E. Busch Blvd. Tampa, FL 33617 813.985.4600 www.adams-engineers.com Mechanical, Electrical Engineer:

Hahn Engineering, Inc. 3060 South Dale Mabry Highway Tampa, FL 33629 813.831.8599 www.hahneng.com

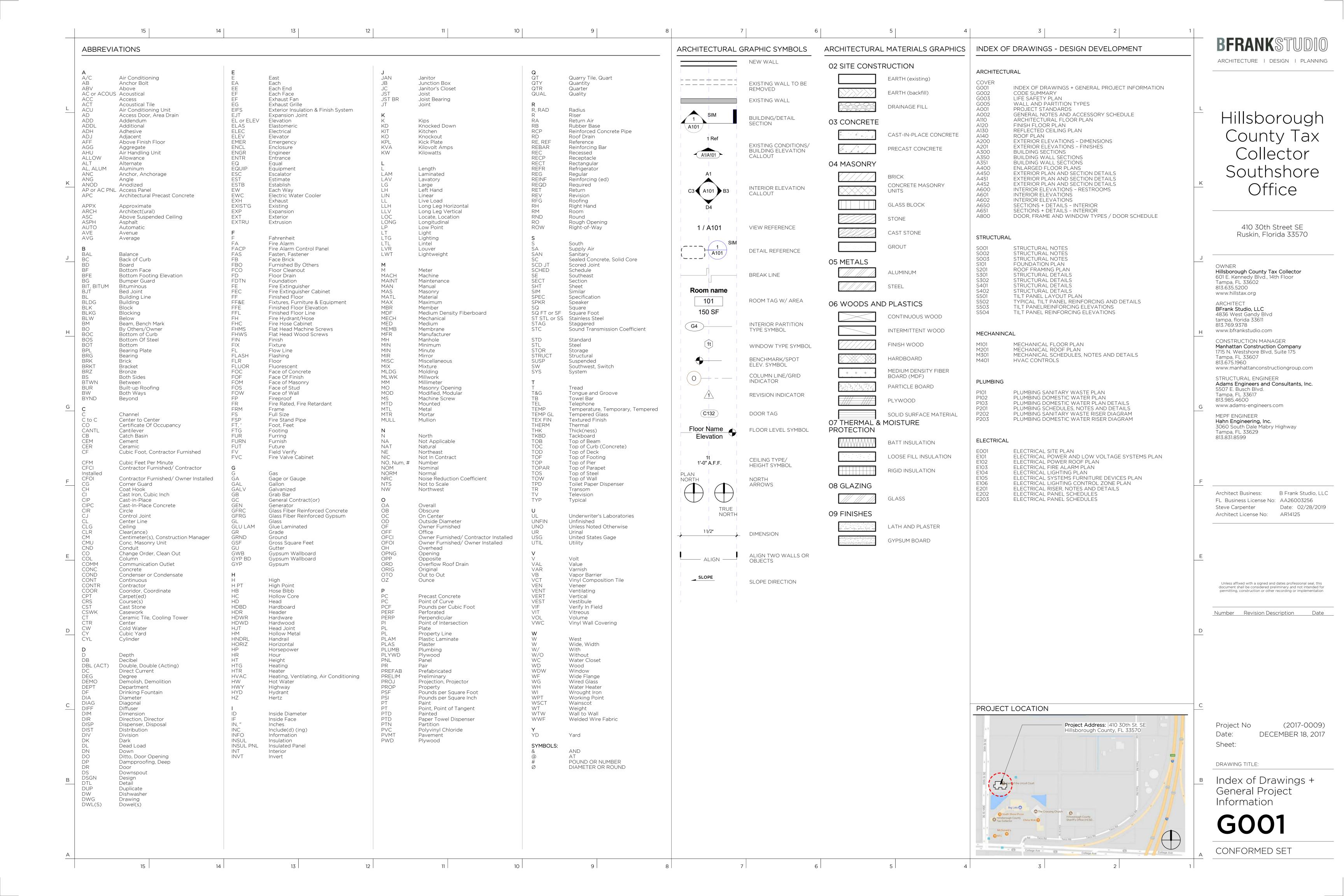


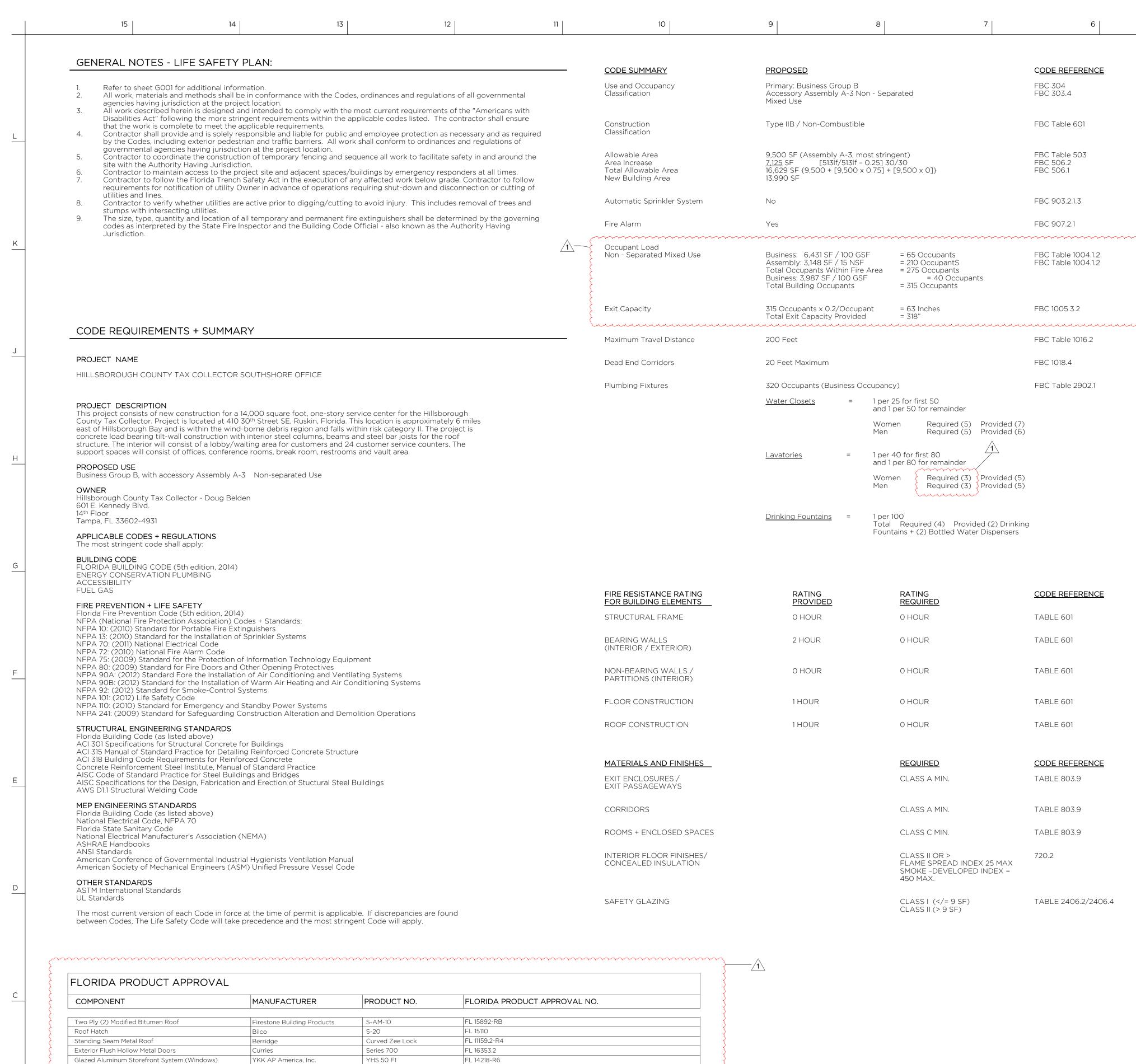
DECEMBER 18, 2017 CONFORMED SET

Project No.

(2017-0009)







150 MPH WIND-BORNE DEBRIS Infrastructure & Development Services **Pinellas** County 410 30th Street SE Ruskin, FL 33570 LEGEND Outside 150 MPH Area 150 MPH Area Municipality Plant City Tampa 601 E Kennedy Blvd, Tampa, FL 33602 (813) 272-5810 Temple Terrace printroom@hillsboroughcounty.org Date: 02/25/2014 Path: W:\GIS\PROJECTS\WindborneDebris\Maps\Mxd\Current\WD_150MPH_AP.mxd

ARCHITECTURE | DESIGN | PLANNING

Hillsborough County Tax Collector

> 410 30th Street SE Ruskin, Florida 33570

OWNER Hillsborough County Tax Collector 601 E. Kennedy Blvd., 14th Floor Tampa, FL 33602 813.635.5200 www.hillstax.org

ARCHITECT BFrank Studio, LLC 4836 West Gandy Blvd tampa, florida 33611 813.769.9378 www.bfrankstudio.com

Manhattan Construction Company 1715 N. Westshore Blvd, Suite 175 Tampa, FL 33607 813.675.1960 www.manhattanconstructiongroup.com

CONSTRUCTION MANAGER

STRUCTURAL ENGINEER Adams Engineers and Consultants, Inc. Tampa, FL 33617

813.985.4600 www.adams-engineers.com MEPF ENGINEER

Hahn Engineering, Inc. 3060 South Dale Mabry Highway Tampa, FL 33629 813.831.8599

B Frank Studio, LLC Architect Business: FL Business License No: AA26003256 Date: 02/28/2019 Steve Carpenter

Architect License No: AR14125

Unless affixed with a signed and dates professional seal, this

permitting, construction or other recording or implementation

ASI-01 - CODE

12.18.2017

Number Revision Description

(2017-0009) Project No **DECEMBER 18, 2017** Date: Sheet:

DRAWING TITLE:

Code Summary

CONFORMED SET

Series 35H

YKK AP America, Inc.

FL 16554-R4

Glazed Aluminum Entrance Doors (Outswinging)

STRUCTURAL NOTES

GOVERNING CODES:

2. DESIGN LOADS:

This design is based on the following codes:

D. Structural Welding Code D1.1

A. Florida Building Code 2014 (5th Edition)

1. Mean Roof Height of 15'-0"

Exposure Category 'C'.

Building Code Requirements for Structural Concrete ACI 318

calculation for wind pressures and the following factors:

Base Velocity Pressure of 38.8 PSF

Vx=32.9 Kip

Vy=35.0 Kip

Ground Snow load of 0 PSF

Roof dead load of 20 PSF.

Allowable soil bearing of 2,000 PSF.

differing from those shown on the drawings.

Base Shear Due to Main Frame Loading

3-1/2" of water can accumulate before entering the overflows.

Specification for the Design, Fabrication, and Erection of Structural Steel for Building, ASD Design method.

Wind load based on ASCE 7-10 "Minimum Design Loads for Buildings and Other Structures" methods of

This building is IN A WIND BORNE DEBRIS REGION as defined by Florida Building Code. All glazing

is assumed to be protected in accordance with Section 1609.1.2.3 of the Florida Building Code.

Roof live load of 20 PSF with allowable load reductions based on area as outlined in the Florida Building

Overflow drains shall be located so that, in the event of the primary drains being blocked, no more than

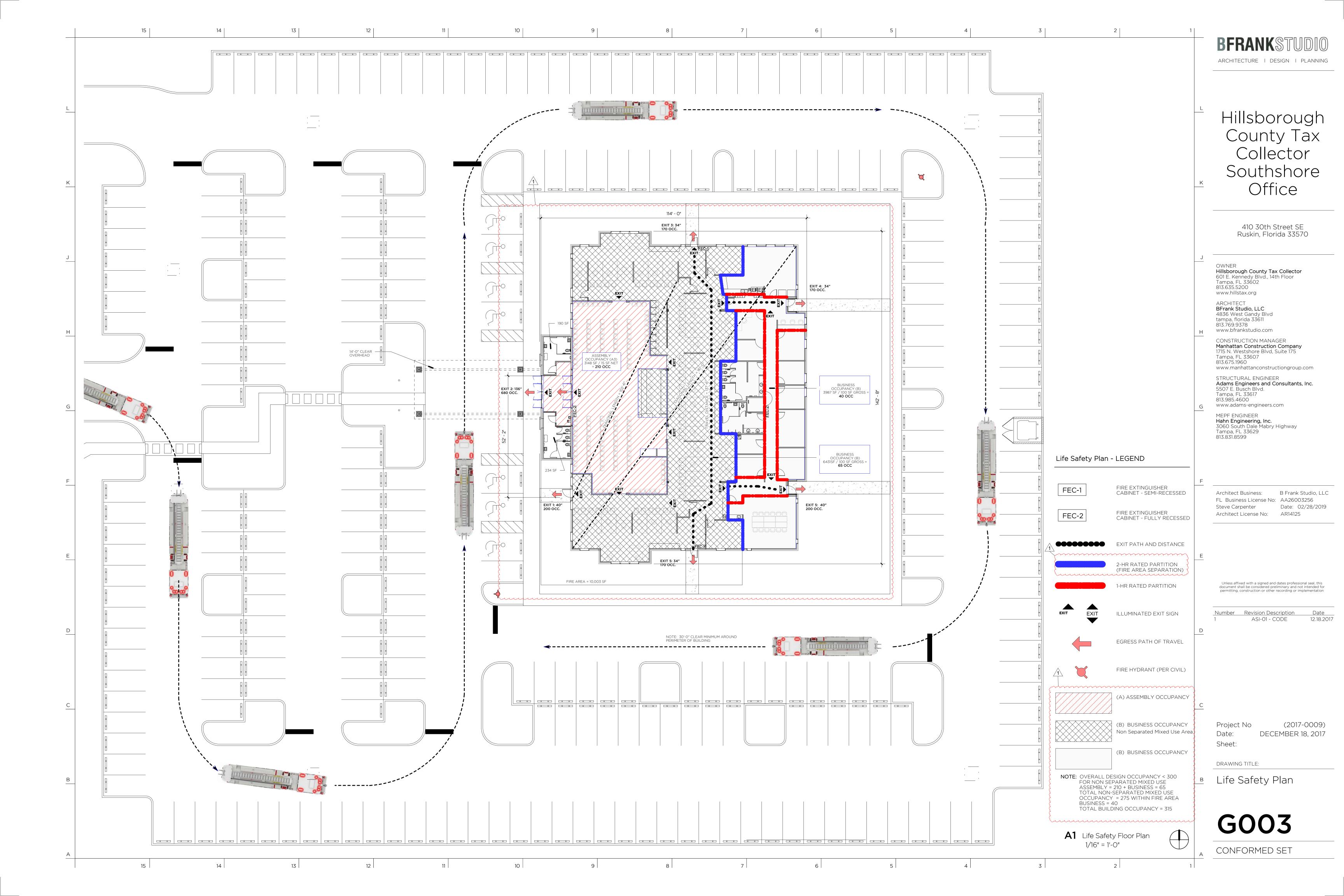
Other loads: Contractor shall submit loading information for all equipment not shown on the drawings, or

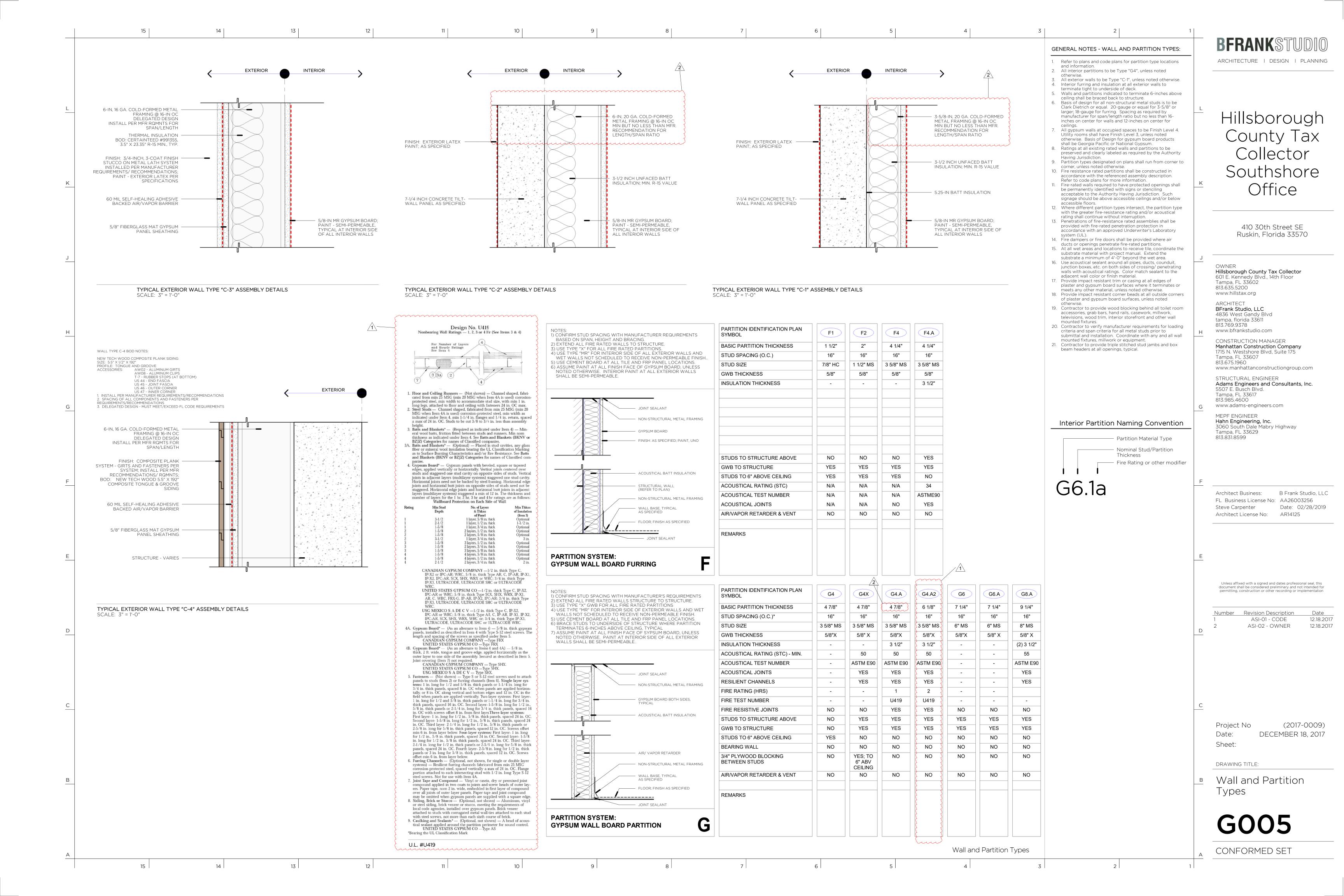
Code. Roof drainage shall conform to the requirements of the Code Section Florida Building Code.

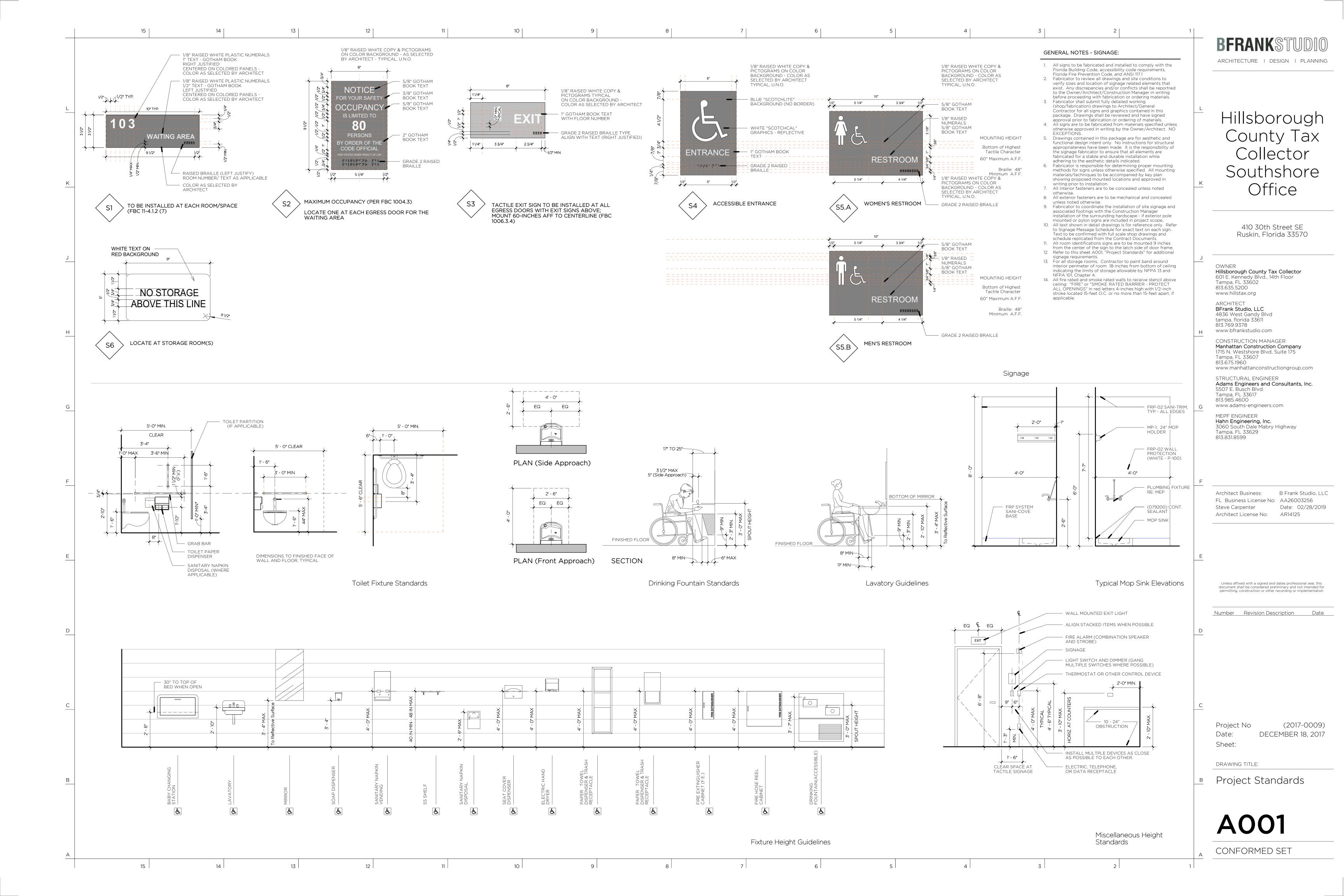
Wind speed of V(ULT)=145 MPH, V(ASD)=112 MPH, Risk Category II

Internal Pressure Coefficient of +/- 0.18 ("Enclosed" building).

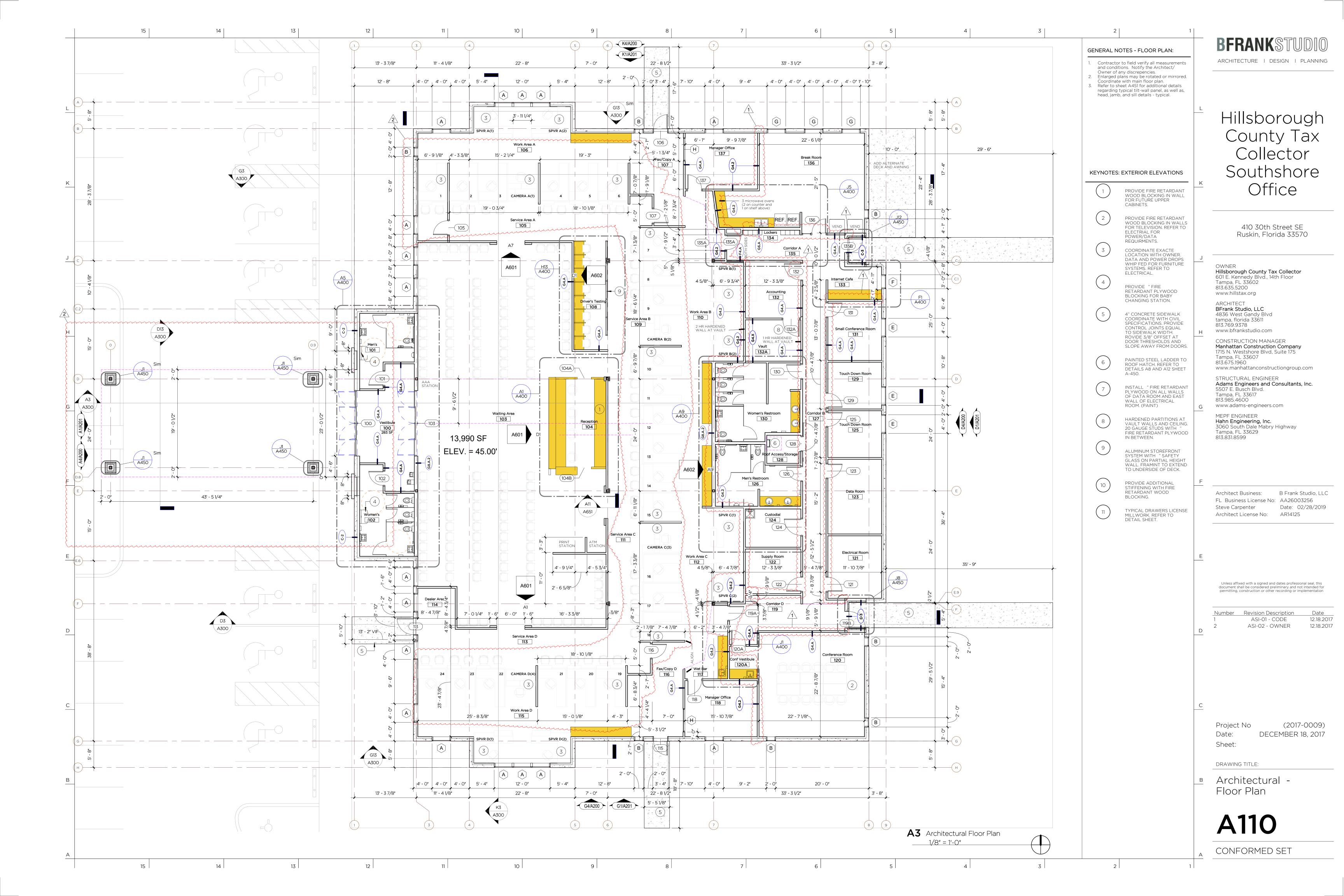
DESIGN CRITERIA

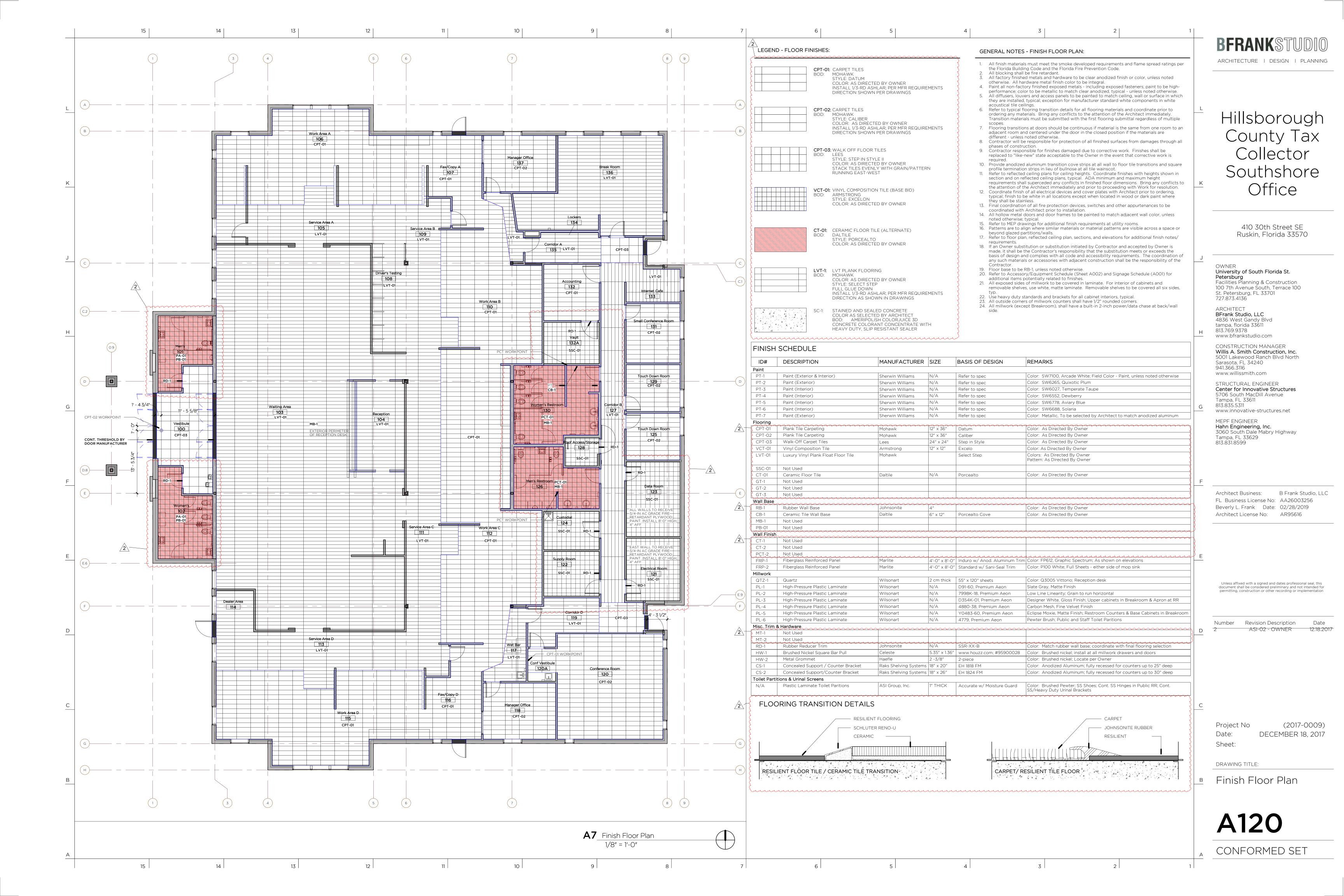


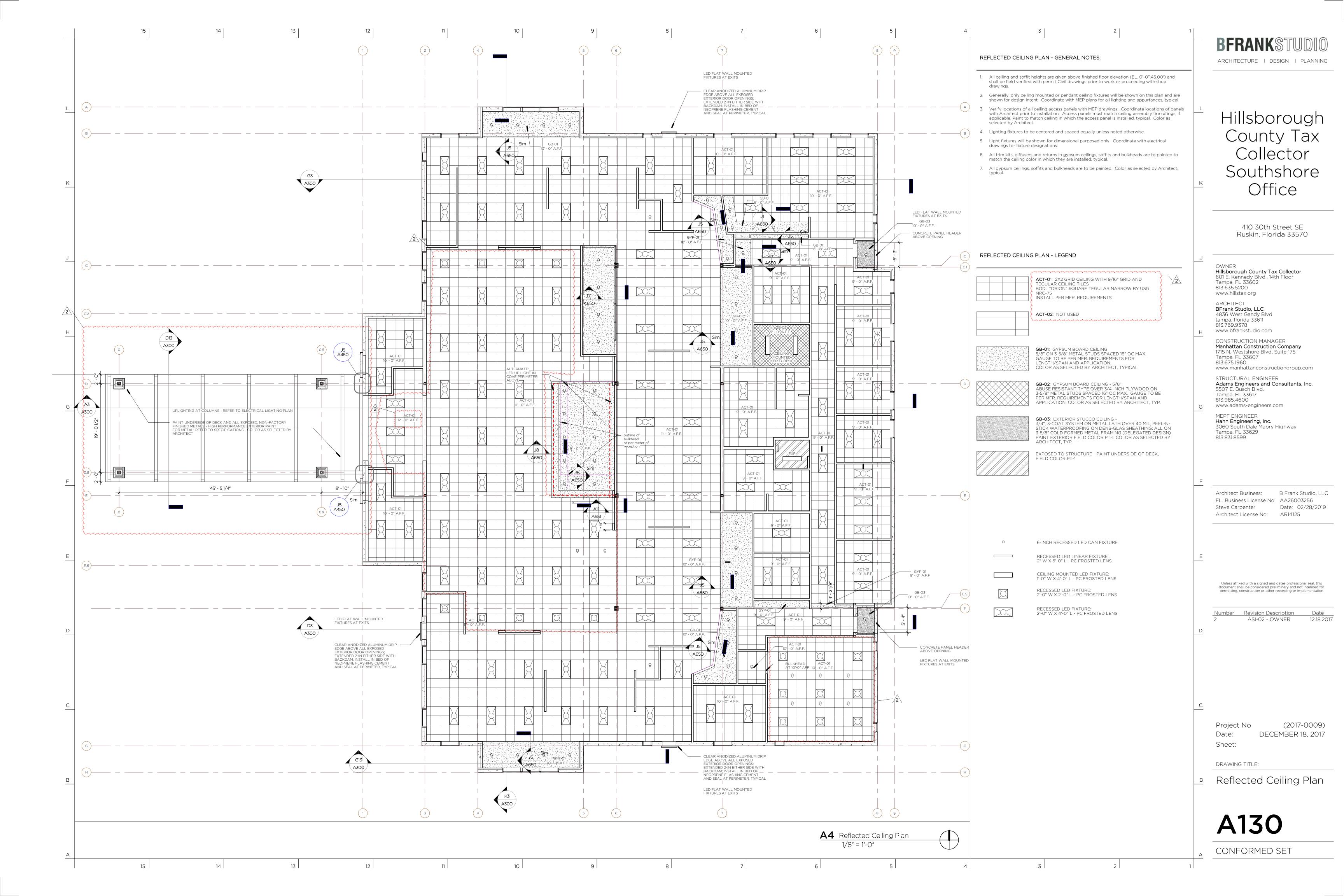


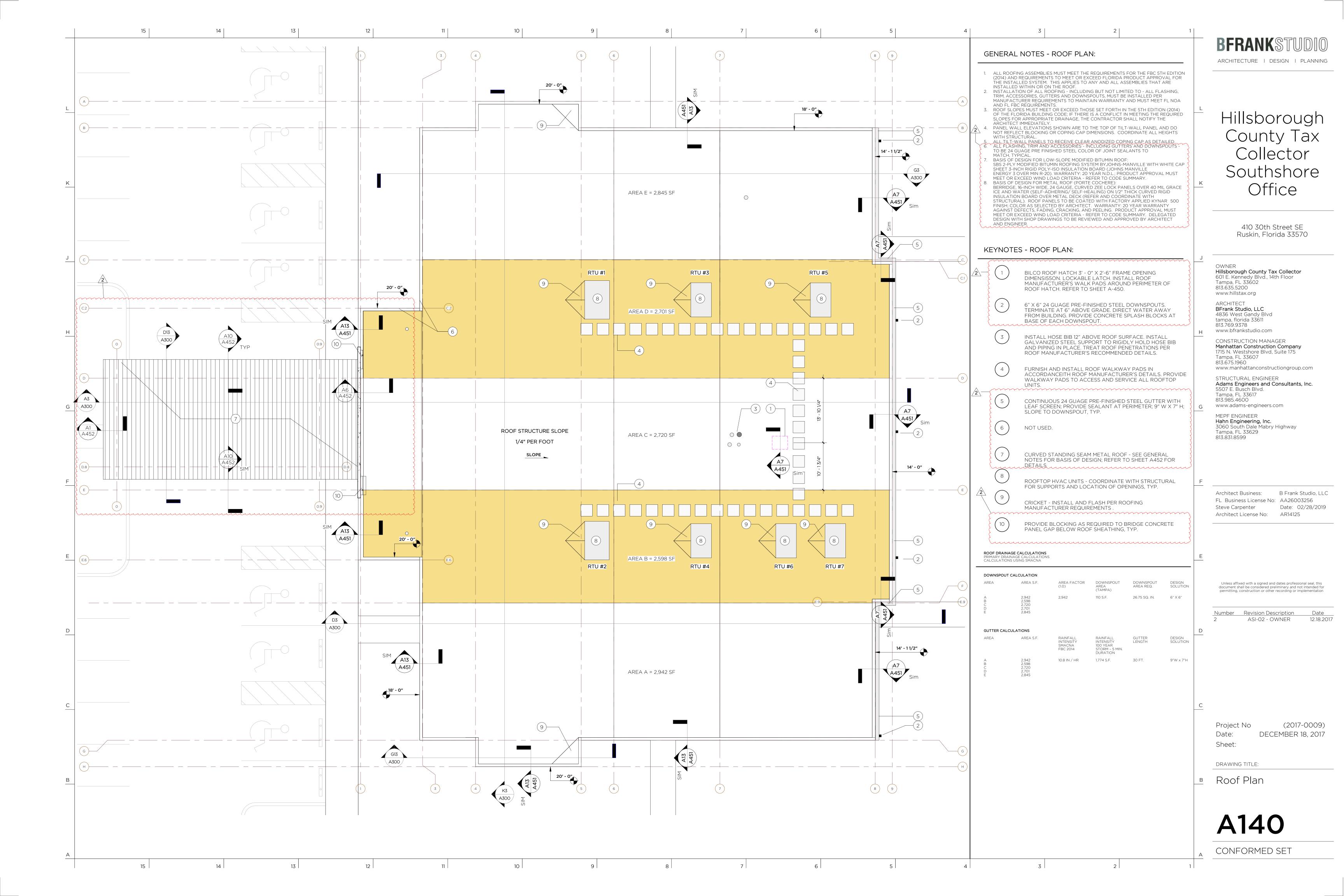


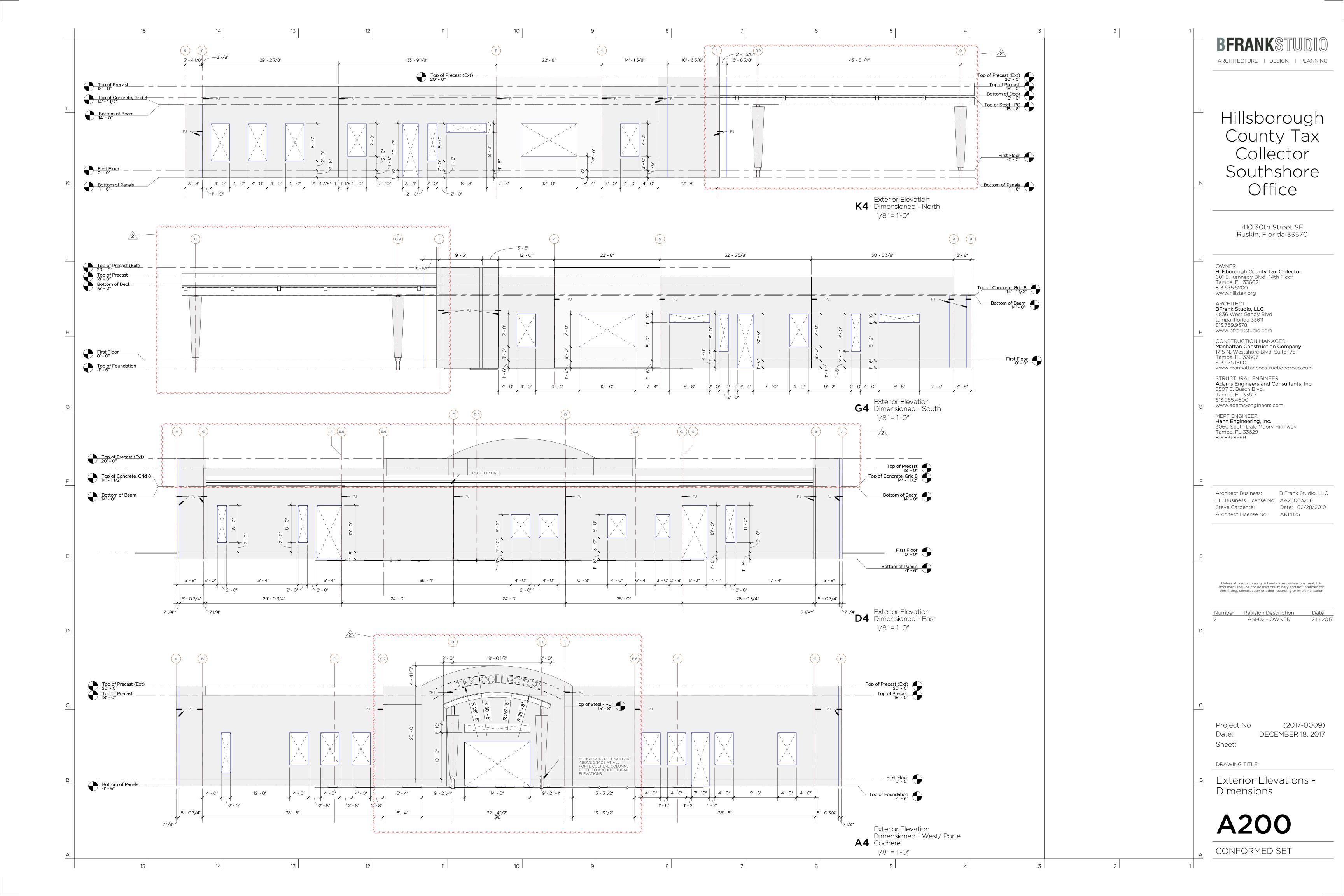
	UIPMENT & ACCESSORY SCHEDULE				GENERAL ARCHITECTURAL DRAWING NOTES: 1. The intent of the Contract Documents is to include all items necessary for proper execution and	BFRAN ARCHITECTURE
ID#	DESCRIPTION	MANUFACTURER	MODEL NUMBER	REMARKS	completion of the Work by the Contractor. The Contract Documents are complementary, and what is required by one shall be as binding as if required by all. Performance by the Contractor shall be required only to the extent consistent with the Contract Documents and reasonably	
Toilet Access BC-1	Baby Changing Station Grab Bars - 36"	Bobrick Washroom Equipment, Inc.	KB-200-00 B-6806.36	Install one each at each public restroom; provide cont. 2x4 stud blocking between studs, typical Install one each at accessible stalls	 inferable from them as being necessary to produce the indicated results. 2. Organization of the specifications into divisions, section, and articles, and arrangement of drawings shall not control the Contractor in dividing the work among subcontractors or in 	
GB-2	Grab Bars - 36" Grab Bars - 42" Hand Dryer - Electric	Bobrick Washroom Equipment, Inc. Bobrick Washroom Equipment, Inc. Excel Dryer - XLERATOReco	B-6806.36 B-6806.42 XL-BW-ECO	Install one each at accessible stalls Install one each at accessible stalls Install one at each public restroom and public accessible stall; Install one each at staff restrooms	establishing the extent of the work to be performed by any trade. 3. Drawings, specifications, general and supplementary conditions are essential parts of the contract. In the event of any discrepency between drawing and figures written thereon, the figures, unless	
HD-1 HK-1 MP-1	Purse/Coat Hook Stainless Steel Mop and Broom Holder with Shelf	Bobrick Washroom Equipment, Inc.	B-672 B-239 x 34	Install one at each public restroom and public accessible stall; Install one each at staff restrooms Install one back of toilet partition door at each stall Install one at Custodial Room	obviously incorrect, are to govern over scaled dimensions. In the case of any discrepancy between the drawings and the specifications, the specifications are to govern. If there is a discrepency between large and small scale details, the large scale details are to govern.	[_] Hillsb
MR-1	Stainless Steel Mop and Broom Holder with Shelf Glass Mirror Stainless Steel Shelf	Bobrick Washroom Equipment, Inc. Bobrick Washroom Equipment, Inc. Bobrick Washroom Equipment, Inc.	B-165 2436	Install one centered above each lavatory, typical	Supplementary conditions shall govern over specifications, drawings and general conditions. The Contractor shall advise the Architect of any discrepancies or conflicts between Contract Documents as soon as they are discovered.	
MS-1 PTD-1	Semi-recessed Paper Towel Disp./ Trash Receptacle	Bobrick Washroom Equipment, Inc.	B-3942	Install one at each staff above water closet and urinal Install one at each staff restroom & each staff accessible stall; do not install in public restooms	4. Notwithstanding the above, in the case of inconsistency between drawing and specifications, or within either document not clarified by Addendum or by Architect's Supplemental Instruction, the	Cour
PTD-2 SD-1	Surface-Mounted Soap Dispenser	Bobrick Washroom Equipment, Inc. Bobrick Washroom Equipment, Inc.	B-2111	Install one at breakroom; install one at custodial; install one at each wet bar Install one per lavatory, typical; install one at breakroom; install one at custodial; install one at each wetbar	better quality or greater quantity shall be provided. 5. Whenever Contract Documents reasonably imply materials or installation as necessary to produce the intended results, but do not fully detail or specify such materials, the contractor shall provide	Col
SN-1 TP-1	Surface-Mounted Sanitary Napkin Disposal Multi-Roll Toilet Tissue Dispenser	Bobrick Washroom Equipment, Inc. Bobrick Washroom Equipment, Inc.	B-265	Install one at each women's restroom stall Install one at each toilet room stall - public restrooms & staff restrooms	the materials and labor required for installation nonetheless, including specific Manufacturer's recommendations and requirements for assembly, execution and installation. 6. Work shall be installed so as not to void the Manufacturer's warranty or the warranty of any	
TS-1	Toilet Seat Cover Dispenser	Bobrick Washroom Equipment, Inc.	B-221	Install one at each toilet room stall - public restrooms & staff restrooms	adjoining or adjacent products or assembly. The Contractor shall notify the Architect immediately if conditions exist within a detail or assembly that could potentially void the warrantly prior to proceeding with the Work.	Sout
NOTE:	ALL ACCESSORIES TO BE CONTRACTOR FURNISHED AND CO	ONTRACTOR INSTALLED, UNLESS NOTED C	OTHERWISE. COORDINATE WITH	OWNER.	signed and sealed shop drawings in a delegated design for the assembly where required for any exterior building assembly. 8. Manufactured assemblies (such as railings and storefront) where loading requirements are	
ID#	DESCRIPTION	MANUFACTURER	MODEL NUMBER	REMARKS	mandated by Code requirements shall be engineered and installed per signed and sealed shop drawings. 9. In the event of a substitution of deviation from the basis of design, the Contractor shall be responsible for ensuring the components meet all building Code requirements and all requirements	
Specialty Equ		Global Equipment Co. or equal	10 lb.; universal	(2) - Install as shown on Life Safety Plan; stainless steel with glass front and roll latch; ADA compliant	needed to maintain the Manufacturer warranty for any portion of the completed work. The Contractor shall also ensure that all components for the substituted assembly are included to achieve the intended results.	410 30 Ruskin,
FEC-1	Fully-Recessed Cabinet and Fire Extinguisher	Global Equipment Co. or equal	10 lb.; universal	(2) - Install as shown on Life Safety Plan; stainless steel with glass front and roll latch; ADA compliant (2) - Install as shown on Life Safety Plan; stainless steel with glass front and roll latch; ADA compliant	10. Abbrevations, material legends, and graphic symbols indicated on this sheet are specific to the Architectural drawings and specifications only. Refer elsewhere in the Contract Documents for information specific to other disciplines.	Ruskir
Technology	LCD Correct TV				 Verify dimensions and existing conditions before commencing work. Report discrepencies to the Architect prior to proceeding with affected work. Building floor plan dimensions are referenced from structural grid. Contractor shall coordinate with Civil scope prior to proceeding with the Work. 	 OWNER
TV65	LCD Smart TV			OFCI with mount; quantity = 9; coordinate locations with Owner for blocking where required	 13. Dimensions are referenced from structural grid, face of concrete/precast, face of masonry, or face of finished surface - unless noted otherwise. 14. Reflected ceiling plan dimensions are referenced from finished surfaces - unless noted otherwise. 	Hillsborough Cou 601 E. Kennedy E
					Ceiling heights are dimensioned from finish floor to finished ceiling height. 15. Casework, plumbing fixtures, toilet partitions, and other fixtures and equipment are dimensioned from finished surfaces - unless noted for the surfaces.	Tampa, FL 33602 813.635.5200 www.hillstax.org
					 16. Dimensions noted as "verify in field", "field verify" or "VIF" shall be checked at the site by the Contractor and reviewed with the Architect before incorporating into the work. 17. Surfaces indicated to "ALIGN" shall align to finish surfaces and shall override dimensions, except in instances where Code and/or accessible clearances are required. Bulkheads and soffits adjacent 	ARCHITECT BFrank Studio, LL
					to walls are intended to align unless noted otherwise. In the event it appears alignment will be off - particulary due to existing conditions, review with Architect before proceeding. In any event,	4836 West Gandy tampa, florida 336
					the Contractor shall notify the Architect of discrepency before proceeding with the Work. 18. Dimensions noted as "HOLD" or "CLEAR" require specific coordination between disciplines and/or manufacturers and shall be maintained without variance.	813.769.9378 H www.bfrankstudio.
					19. Drawings noted as "N.T.S." are not to scale and are greater or lesser than the physical relationship shown.20. Do not scale drawings. Written dimensions take precedence. If clarification is required in order to	CONSTRUCTION M Manhattan Constru
					determine the intent of the contract documents, contact the Architect prior to incorporating into the Work. 21. Notes or dimensions labeled "TYPICAL" shall apply to situations that are the same or similar.	1715 N. Westshore Tampa, FL 33607
					22. Notes, sections, details or elevations labeled "SIM" shall be similar in intent to those referenced. If clarification is required in order to determine the intent of the contract documents, contact the Architect prior to incorporating into the Work.	813.675.1960 www.manhattanco
					23. These notes are not intended to limit the responsibilites of the Contractor as defined elsewhere in the Contract Documents.	STRUCTURAL ENC Adams Engineers
					24. Furniture shown depicts intent. Contractor shall coordinate with Owner and Owner's subcontractor final furniture plan to ensure power, data, flooring surfaces, and any other requirements are fully coordinated.	5507 E. Busch Blvc Tampa, FL 33617
						813.985.4600 G www.adams-engine
						MEPF ENGINEER Hahn Engineering,
						3060 South Dale N Tampa, FL 33629 813.831.8599
						813.831.8399
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						Architect Business FL Business Licer Steve Carpenter
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					1. Provide galvanic protection between dissimilar materials. 2. Install piping and conduit tight to walls, columns and roof deck. 3. Seal all pope or conduit penetrations with appropriate sealant. Confirm rating of all walls and stencil as required. Provide paintable fire sealant at rated partitions.	FL Business Lice Steve Carpenter Architect License
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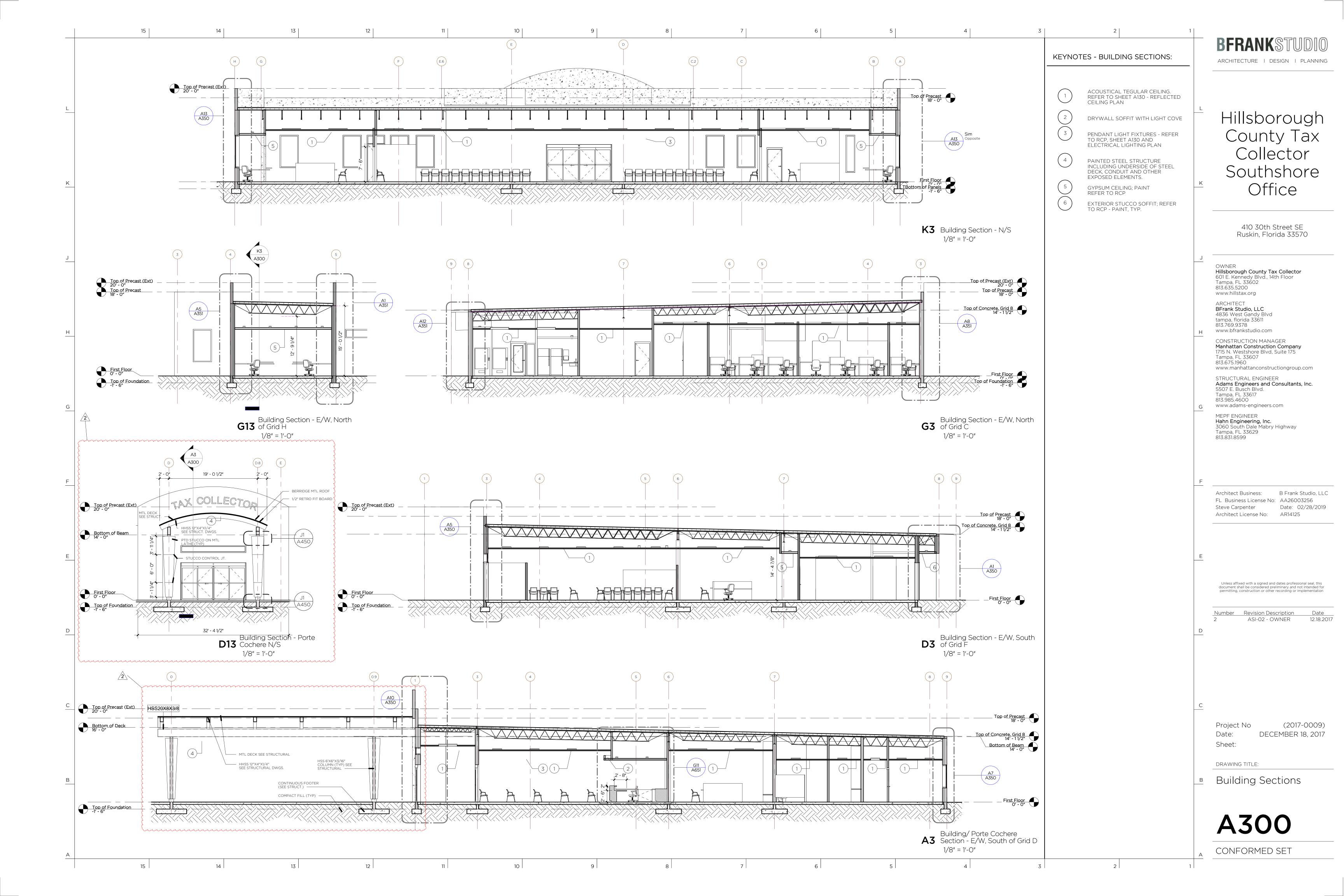


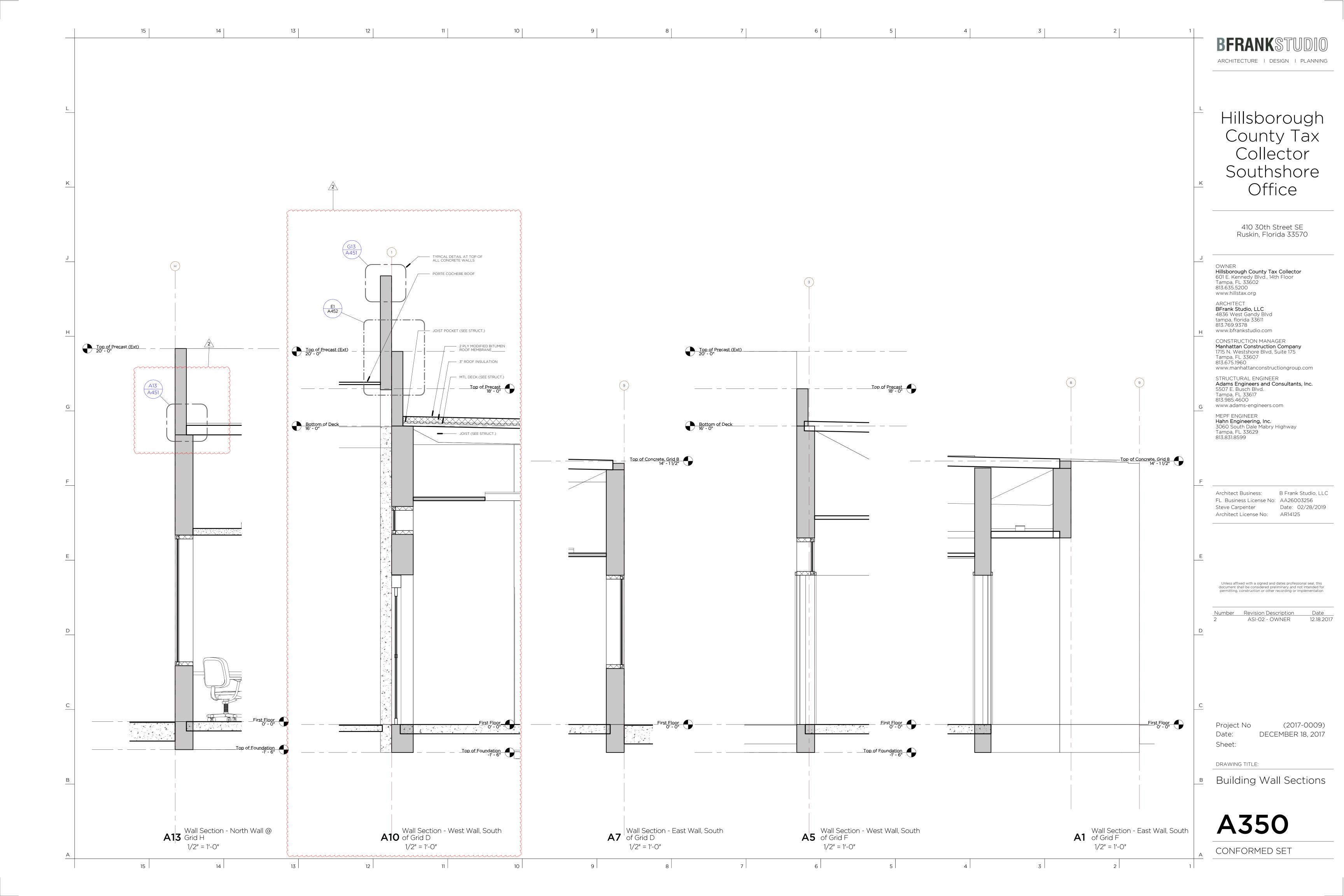


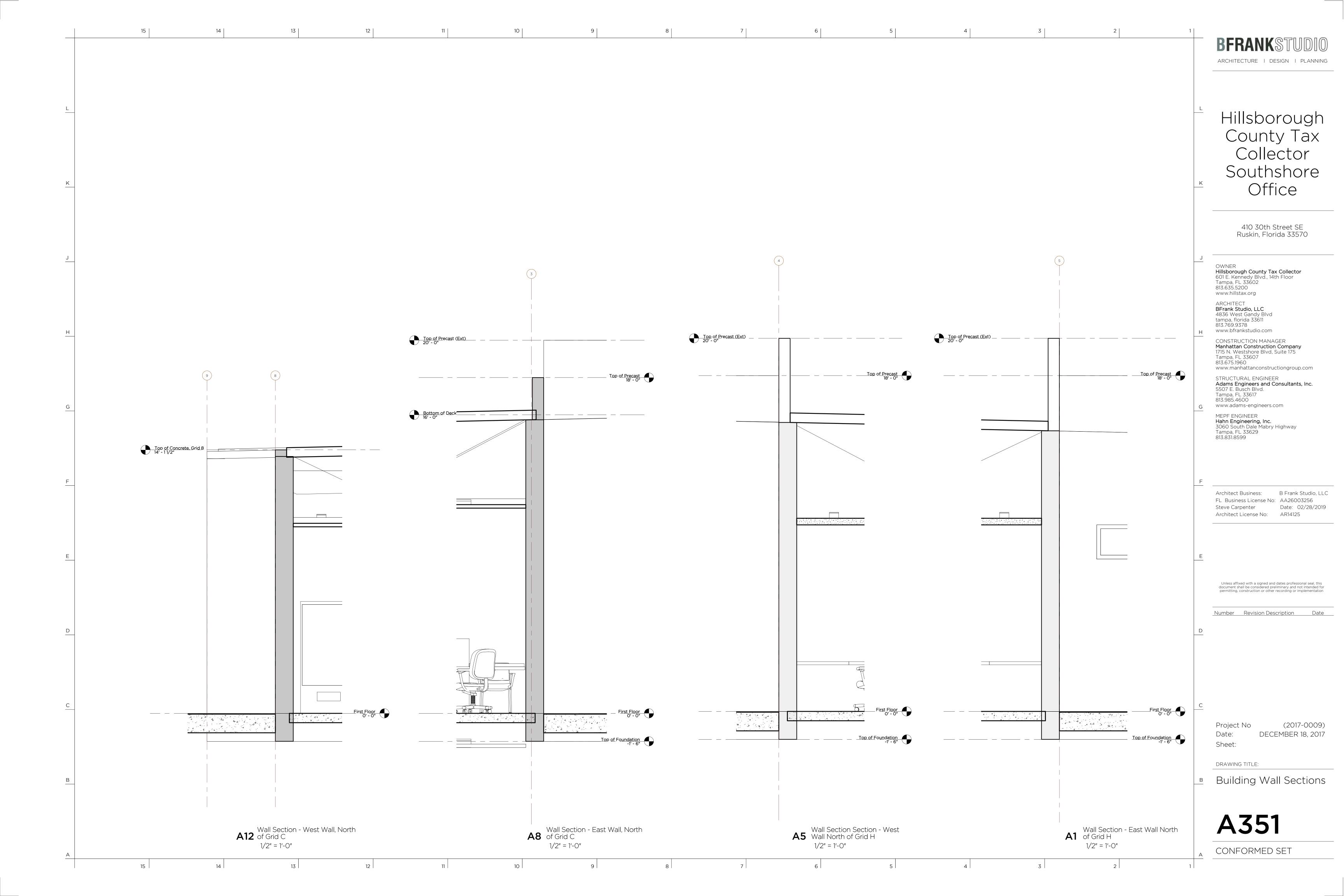


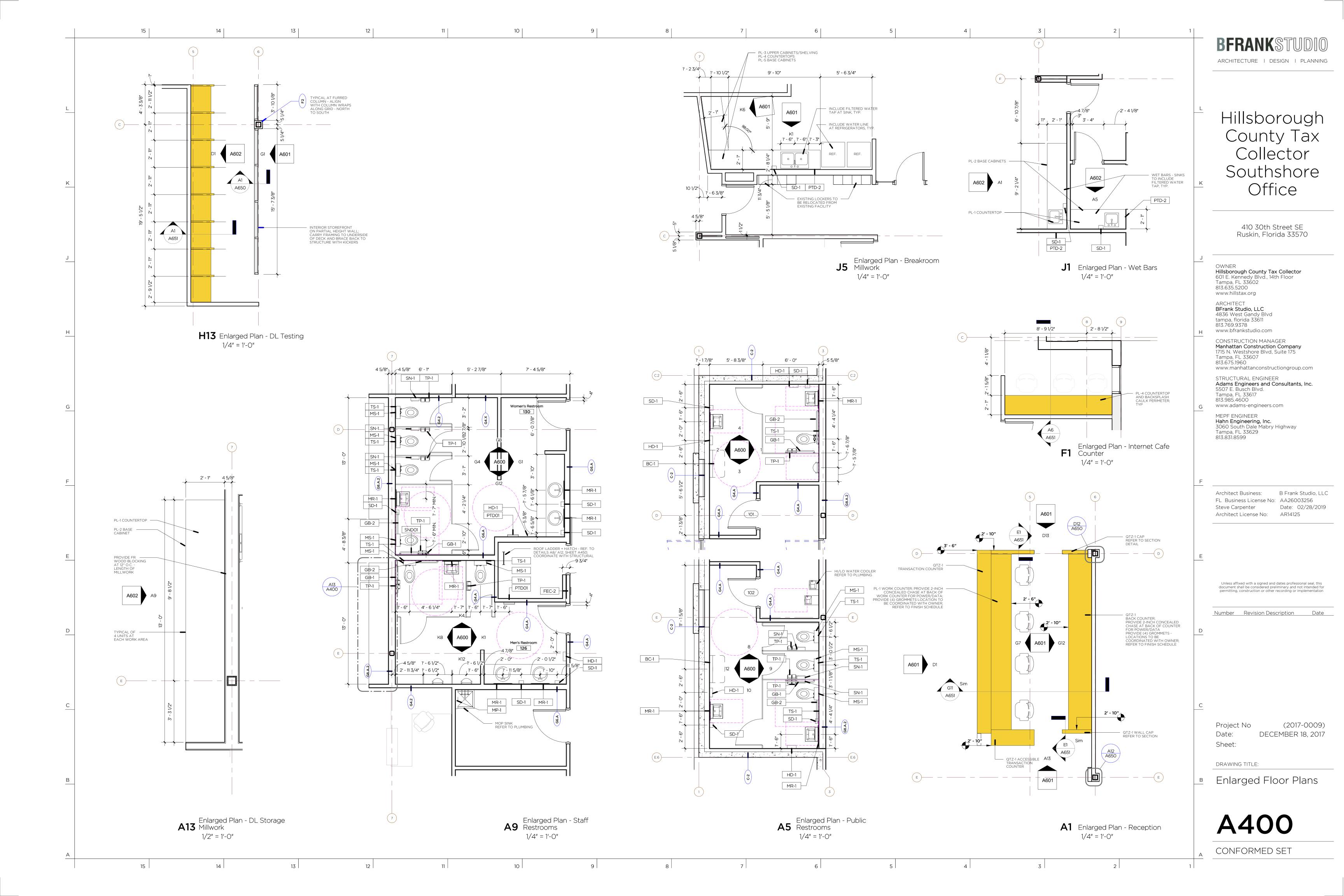


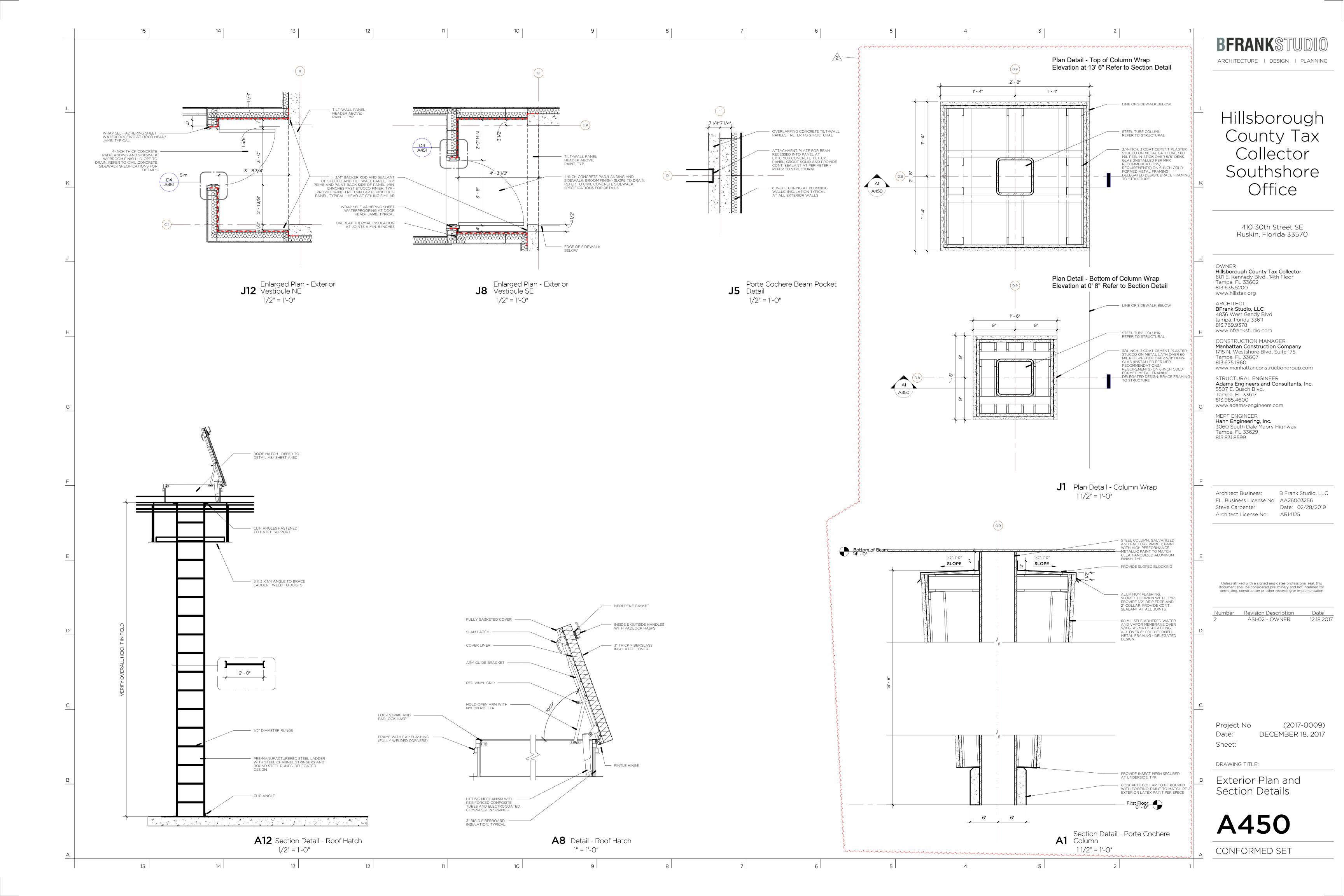


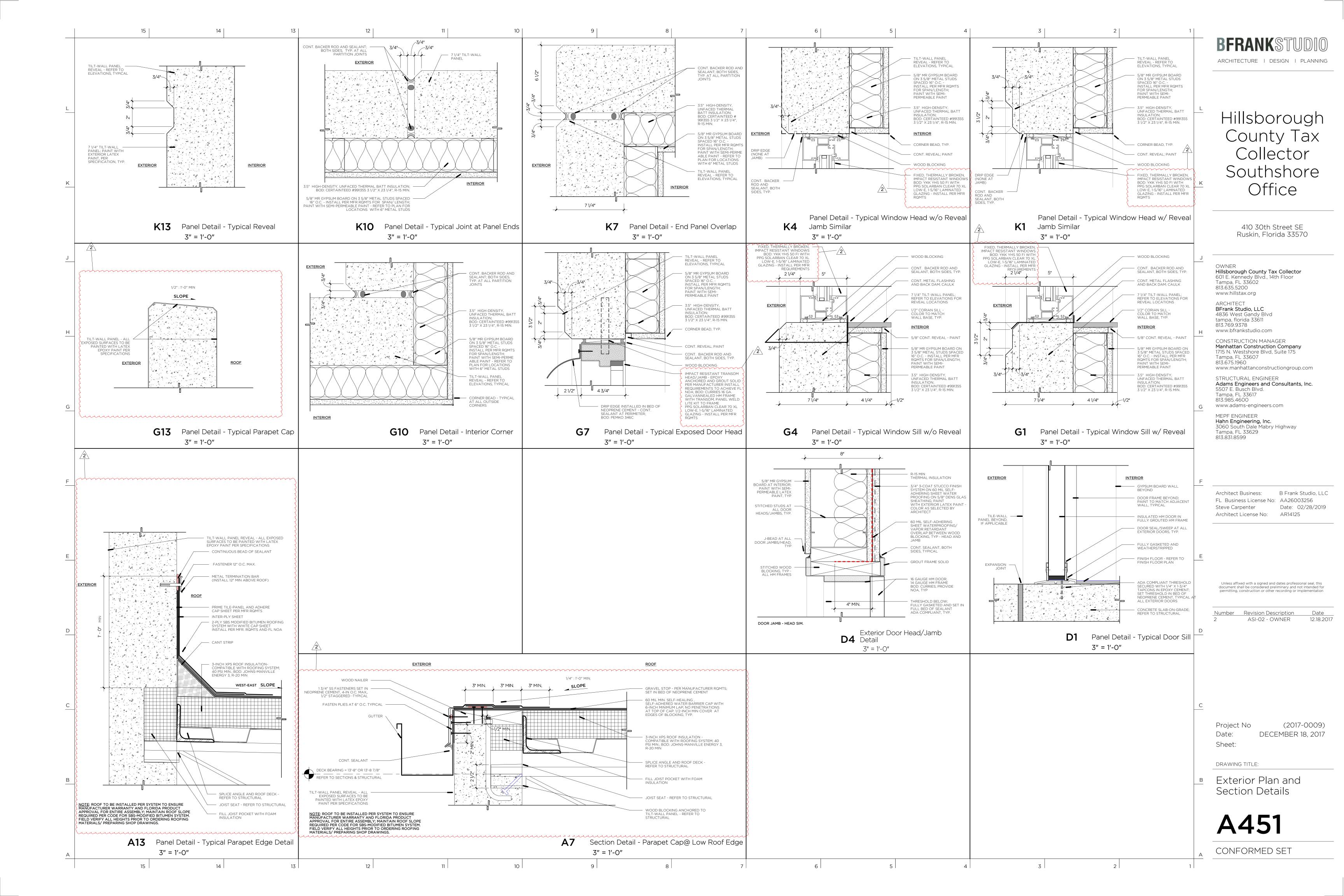


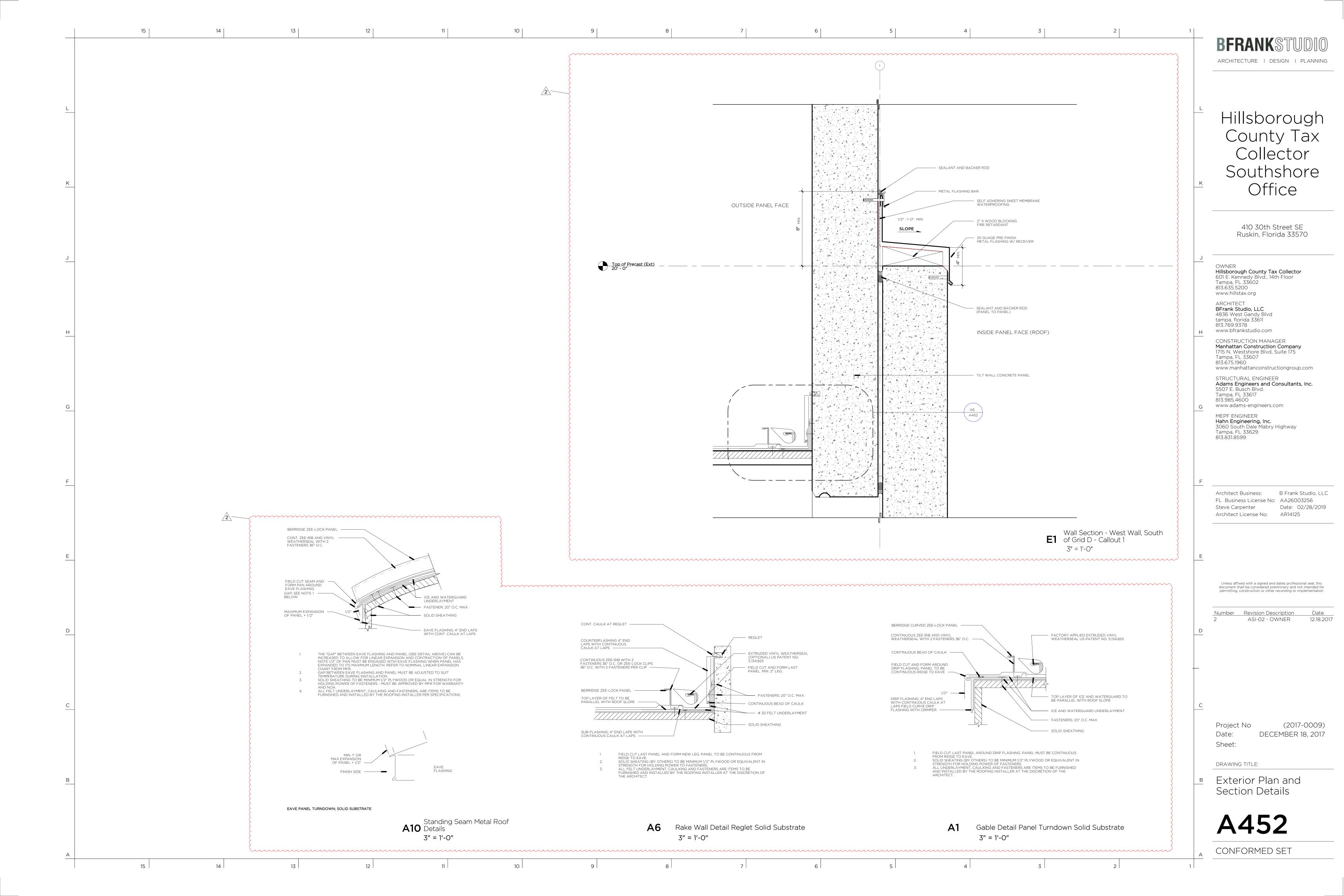


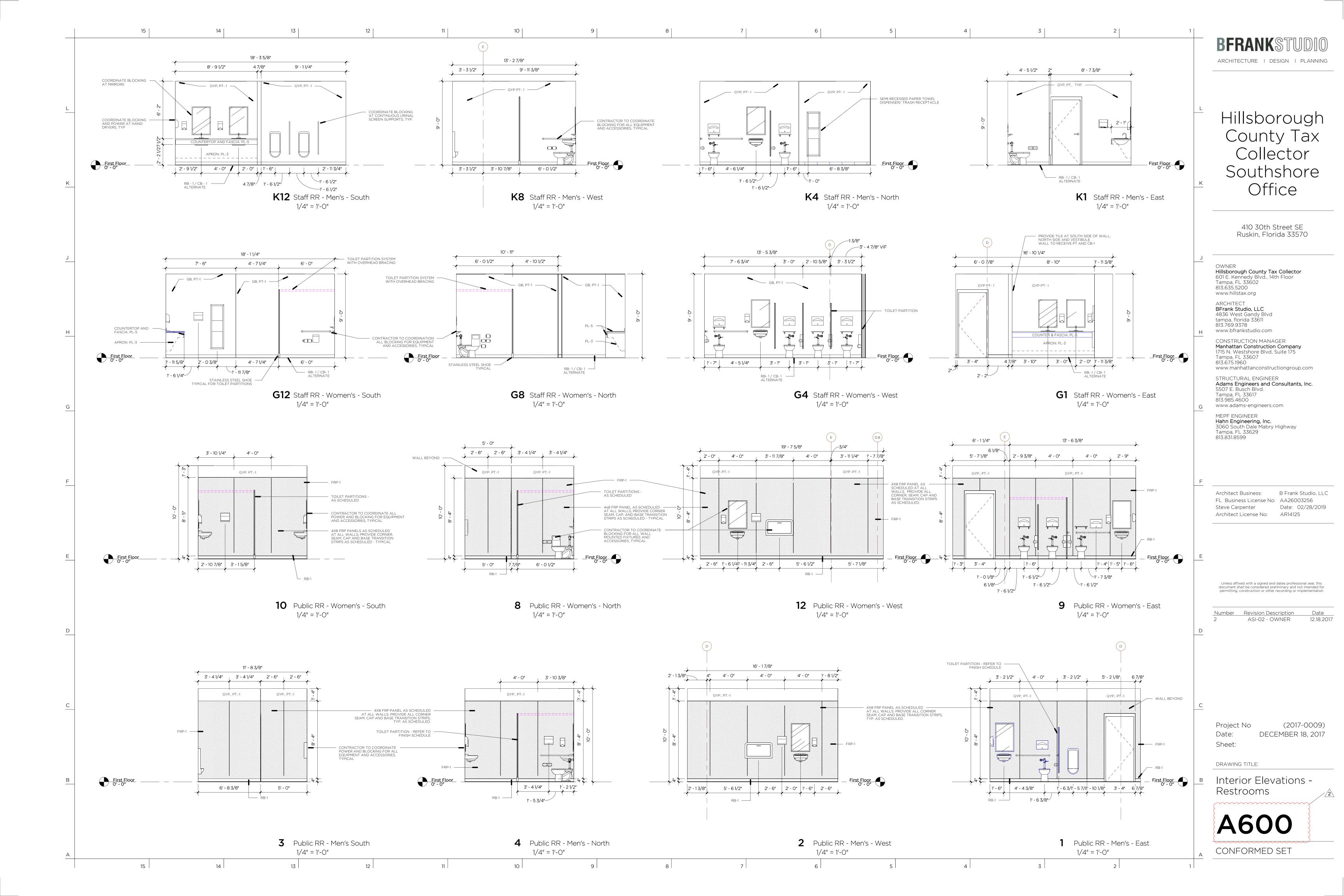


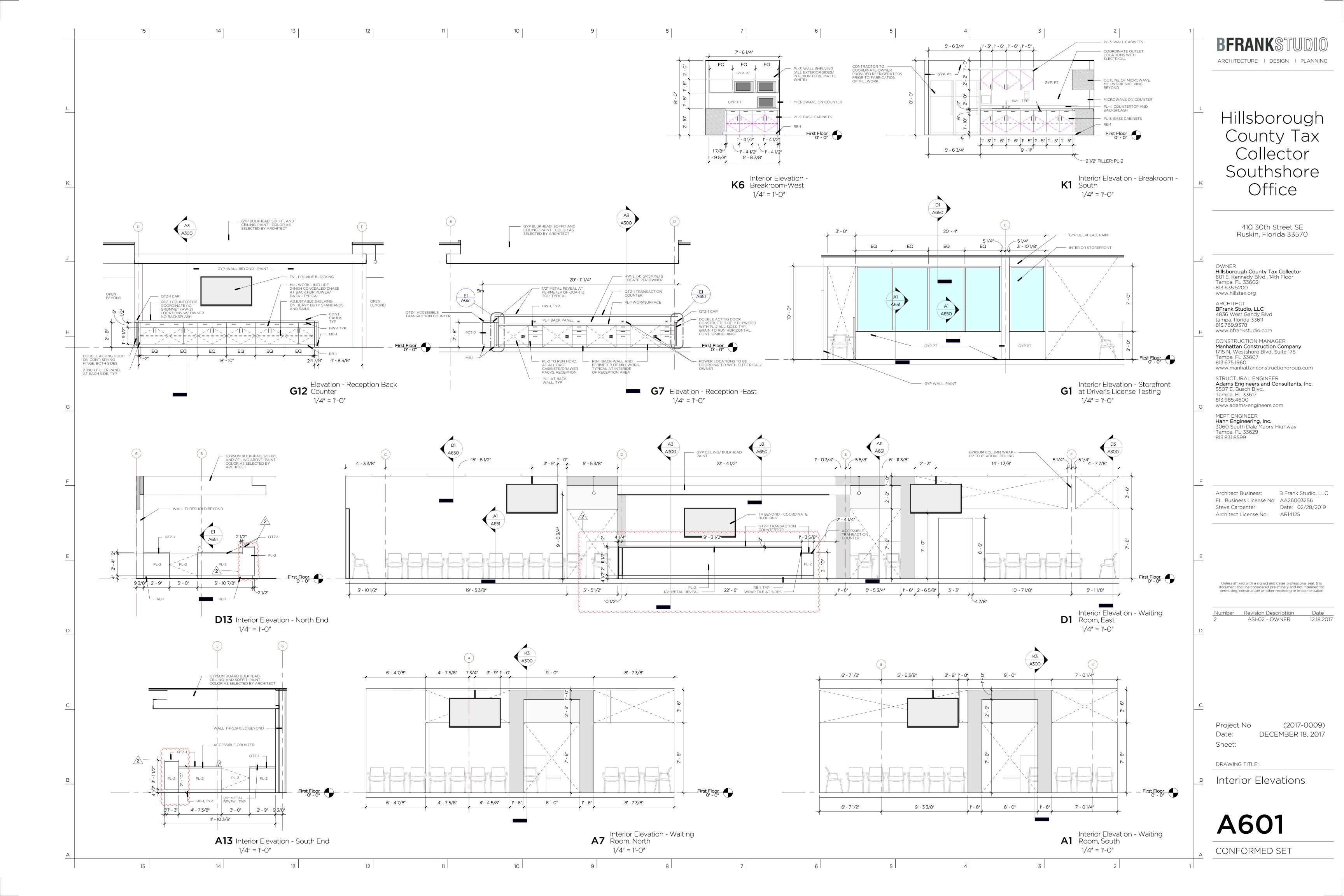


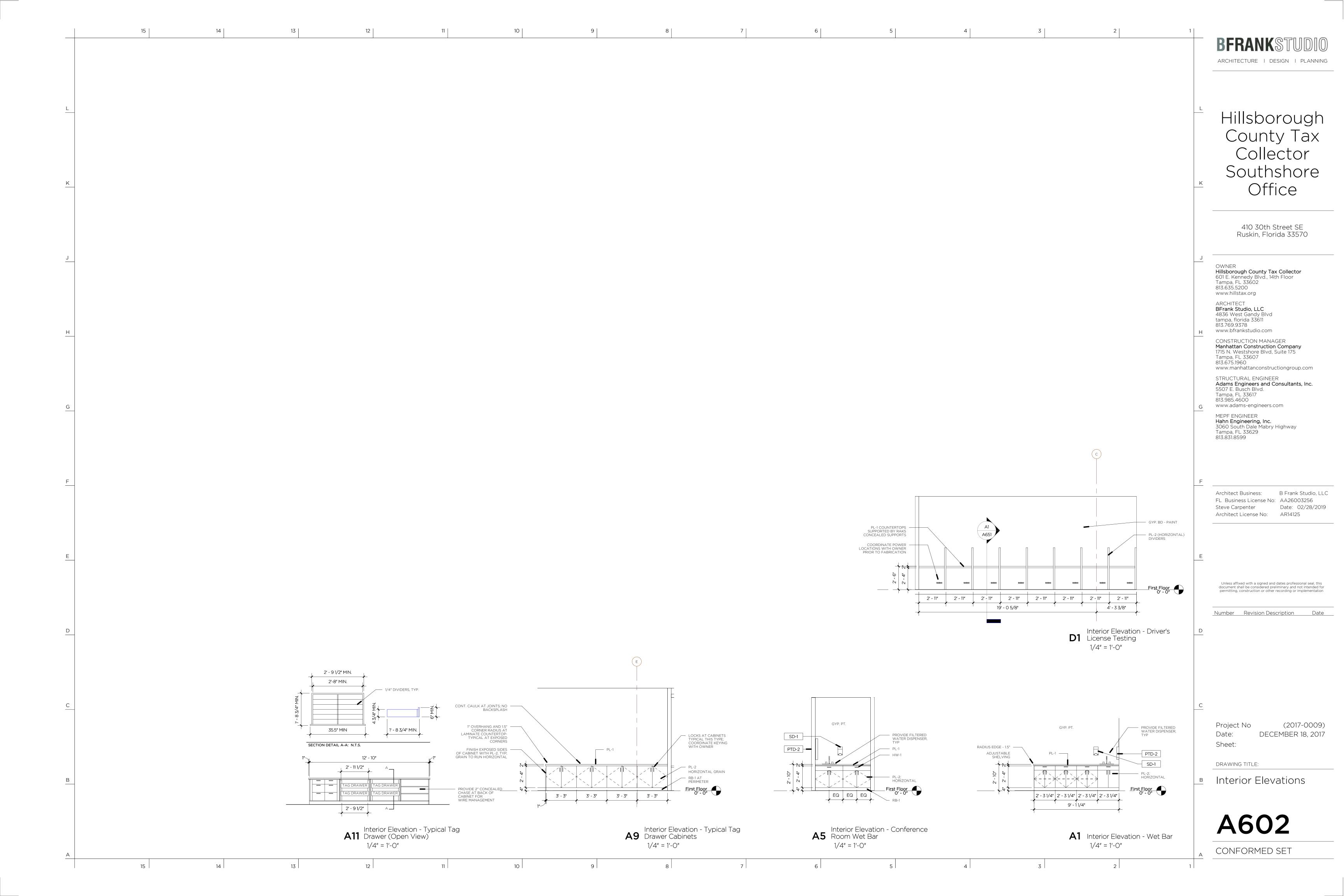


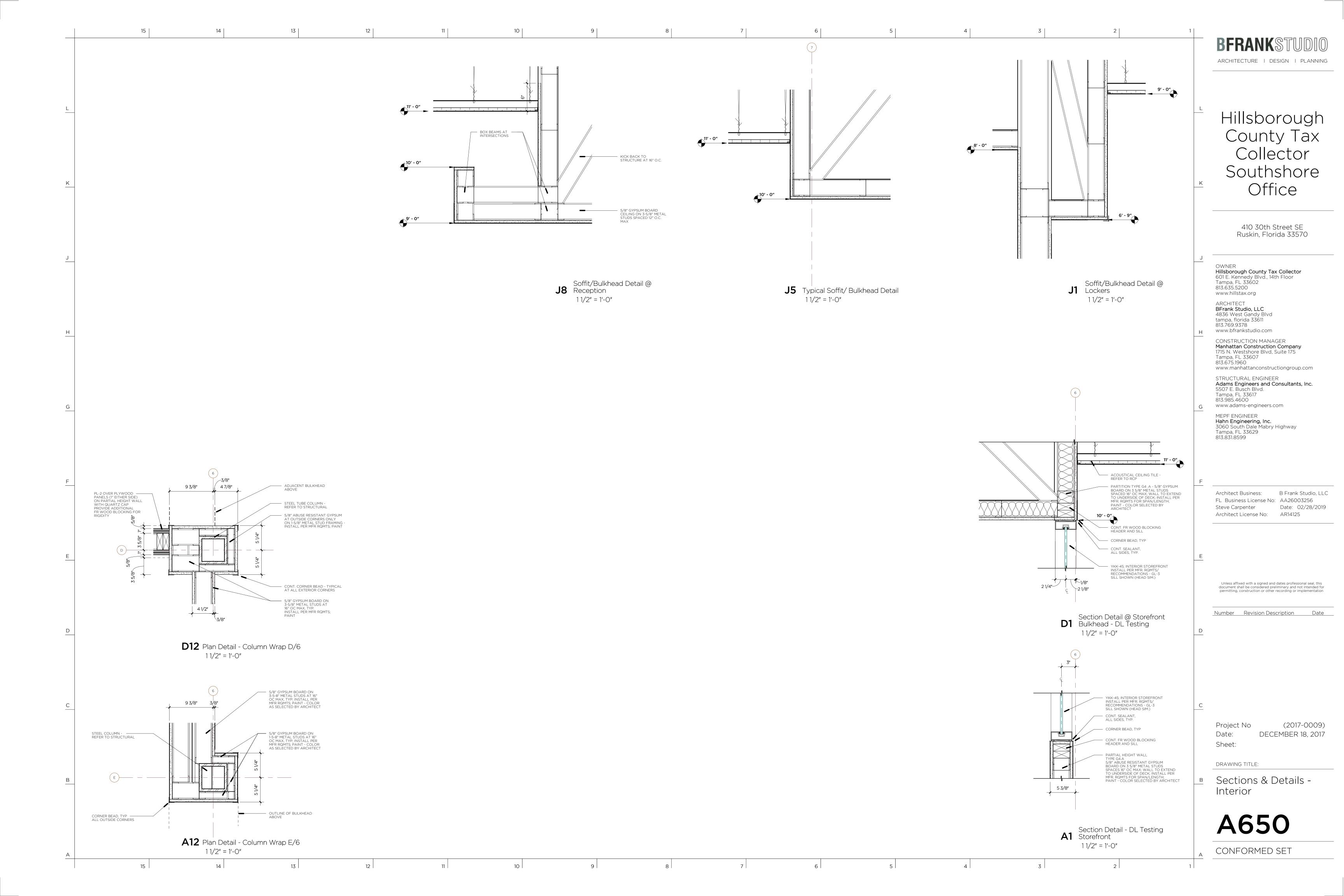


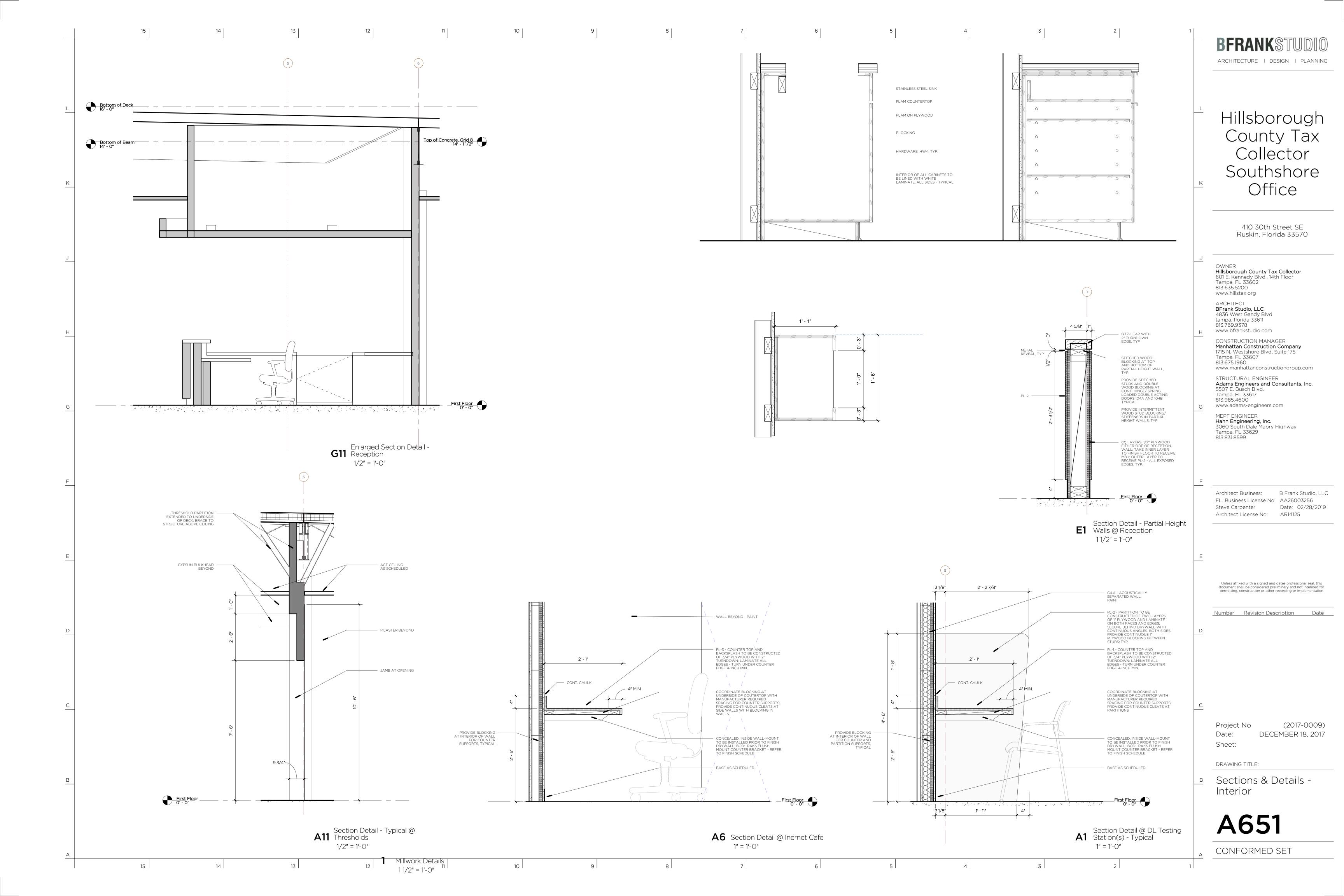


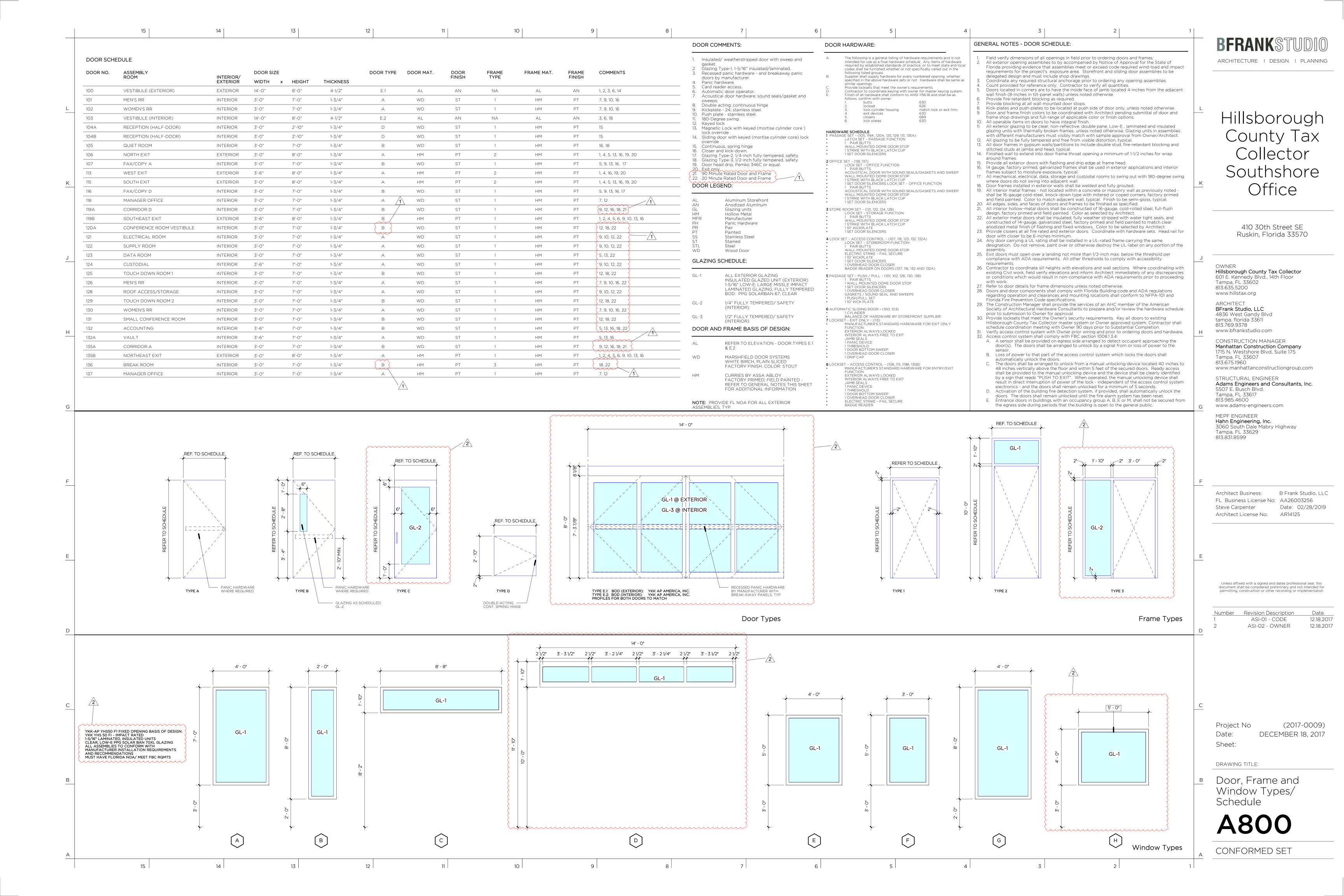












I. GOVERNING CODES:

This design is based on the following codes:

- A. Florida Building Code 2014 (5th Edition)
- B. Building Code Requirements for Structural Concrete ACI 318
- C. Specification for the Design, Fabrication, and Erection of Structural Steel for Building, ASD Design method
- D. Structural Welding Code D1.1

I. DESIGN LOADS:

- A. Wind load based on ASCE 7-10 "Minimum Design Loads for Buildings and Other Structures" methods of calculation for wind pressures and the following factors:
 - 1. Mean Roof Height of 15'-0"
 - 2. Wind speed of V(ULT)=145 MPH, V(ASD)=112 MPH
 - 3. This building is IN A WIND BORNE DEBRIS REGION as defined by Florida Building Code. All glazing is assumed to be protected in accordance with Section 1609.1.2.3 of the Florida Building Code.
 - 4. Exposure Category 'C'.
 - 5. Internal Pressure Coefficient of +/- 0.18 ("Enclosed" building).
 - 6. Base Velocity Pressure of 38.8 PSF
 - 7. Base Shear Due to Main Frame Loading
 - a) Vx=32.9 Kip
 - b) Vy=35.0 Kip
- B. Ground Snow load of 0 PSF
- Roof live load of 20 PSF with allowable load reductions based on area as outlined in the Florida Building Code. Roof drainage shall conform to the requirements of the Code Section Florida Building Code. Overflow drains shall be located so that, in the event of the primary drains being blocked, no more than 3-1/2" of water can accumulate before entering the overflows.
- blocked, no more than 3-1/2" of water can accumulate before entering the overflows.

 D. Roof dead load of 20 PSF.
- E. Allowable soil bearing of 2,000 PSF.

III. SHOP DRAWINGS:

- A. Contractor shall allow for (10) ten business days for shop drawing review.
- B. Our office will accept shop drawings in both electronic and paper format.
- C. If the Shop Drawings are in a paper format, we will require (3) three copies minimum submitted with (1) one copy to be retained by our office.
- D. If the Shop Drawings are in electronic format the following shall apply:
 - 1. The only accepted electronic format shall be Adobe PDF format.
 - 2. If the electronic files are e-mailed to our office, the e-mail address is
 - Mail@Adams-Engineers.com and a faxed transmittal shall be sent to our office informing us that the drawings were sent via e-mail.
 - 3. If the electronic files are sent to our office on a CD or DVD a transmittal shall be included in the package.
 - 4. The reviewed shop drawings shall be returned to Project Architect (or directed party) in either electronic format or paper format as directed by Client. If paper format is requested, it shall be blackline copies with any comments generated by our office in contrasting font or "boxed". Additionally we will invoice for the printing costs associated with the production of these drawings based on the prevailing prints costs in the Tampa Bay Area.

IV. DRAWINGS AND SPECIFICATIONS:

- A. Do not scale these drawings for dimensions that are not given. Advise the Project Architect of any conflicts between these drawings and the Architectural drawings. Verify all field conditions and confirm column locations in respect to architectural wall alignment prior to the start of work.
- B. These drawings are to be used in combination with the architectural, mechanical, plumbing and civil drawings. Refer to these other drawings for details that relate to structural components.
- C. These construction documents have been prepared from the most complete information available to the engineer. All data on existing construction conditions are approximate and shall be verified prior to commencing work.
- D. The contractor shall comply with the manufacturer's instructions and recommendations to the extent-printed information are more detailed or stringent than the requirements contained in the plans.
- The plans show the location of all fixtures and equipment and are intended to convey the general intent of the work in scope and layout. They are not intended to show in minute detail every and all of the accessories intended for the purpose of execution of the work, but it is understood that such details are part of this work.
- F. The Contractor shall not perform any portion of the work at any time without Contract Documents or, where required, approved shop drawings, product data or supplemental details for such portion of the work.
- G. The Contractor is responsible for means and methods of construction to ensure the safety of the building until the structural system is completed. The structural system is unstable until all connections have been made and all concrete has reached the minimum design strength as specified in these drawings.
- H. The use of electronic files or reproductions of these contract documents by any contractor, subcontractor, erector, fabricator or material supplier in lieu of prepared shop drawings signifies their acceptance of all information shown hereon as correct, and obligates themselves to any job expense, real or implied, arising due to any error that may occur hereon.

V. CONCRETE:

- A. Concrete strength requirement:
 - 1. 3,000 psi: Foundations, slab-on-ground
 - 2. 4,000 psi: Tilt wall panels,
- 3. Maximum Slump: 4" (+/- 1") for all concrete, except use 8"-11" for filling cells in block. Slump limits shall be strictly adhered to.
- C. Fly ash, if used, shall not exceed 20% by weight of total cementitious content. Fly ash shall not be used in the Tilt wall panel mix.
- D. The concrete shall contain the maximum size aggregate permitted by ACI Code up to 1 1/2" maximum. The guidelines for maximum aggregate size are not greater than 1/5 the narrowest opening in the forms, and/or 1/3 the depth of the slab.
- E. Concrete formwork: Concrete formwork shall be in clean condition for use as finished surface forms. Forms shall not be removed until the concrete is of sufficient strength to support its own weight and proposed construction loads.
- F. Minimum Concrete Cover
 - 1. Concrete cast against and permanently exposed to earth = 3 inches.
- 2. Concrete exposed to earth or weather = 1-1/2 inches
- 3. Concrete not exposed to earth or weather = 3/4 inches
- G. Concrete must be batched, mixed, and transported in accordance with The Specifications for Ready-Mixed Concrete ASTM C94. Concrete shall be placed with-in 90 minutes of batch time.
- H. The addition of water in the field is acceptable ONLY where the concrete supplier provides a "Water Stamp" on the mix ticket indicating how much additional water can be added to the mix without exceeding the design water / cement ratio.
- I. A qualified testing laboratory shall be retained to perform the following concrete tests. The concrete samples shall be taken in accordance with ASTM C 172 and specimens made in accordance with ASTM C 31. Testing of specimens shall be in accordance with ASTM C 39. Each test shall consist of a minimum of 5 cylinders. One sample shall be tested at 7 days, 14 days, and two at 28 days. Reports showing the results of testing shall be submitted at the earliest possible date following testing and shall indicate date, time, weather conditions (temperature), composition of mix (per delivery ticket), slump, location of concrete, compressive strength, type of break, and other pertinent information helpful in the evaluation of the tests. All tests shall be sequentially numbered for easy identification. The concrete test cylinders shall be taken for each type of class of concrete and not less than once per daily pour, nor less than the following intervals:
 - 1. Footings and sidewalks: One set of samples for each 100 cubic yards of concrete placed, nor less than once per daily pour.
 - 2. Floor slabs: One set of samples for each 50 cubic yards of concrete placed, nor less than once per daily pour.
 - 3. Tilt wall panels: One set of samples for each 50 cubic yards of concrete placed, nor less than once per daily pour.
- J. Concrete Mix submittals shall include any relevant information regarding the use of fiber (Poly and/or Steel) reinforcing that will be added at the batch plant. Fiber shall not be added at the job
- K. The addition of Calcium Sulfate to a mix design to increase the Heat of Hydration to offset cold weather pours where freezing is possible is not permitted.
- L. Concrete pours where freezing is possible shall follow the recommendations of ACI 306.
- M. Expansion and Contraction Joints: Floor (contraction) joints shall be saw cut or prefabricated at a maximum spacing of 2 to 3 times the slab thickness in feet (i.e. 8'-12' o/c for a 4" thick slab). These contraction joints shall be approximately 1/8" wide and extend to a minimum depth of 1/4 the slab thickness. A 1/4" perimeter expansion (isolation) joint shall be used at floors adjoining walls and around all columns. These isolation joints shall be formed by installing an asphalt impregnated fiber sheet that extends the entire thickness of the slab.

VI. UNDERSLAB VAPOR BARRIER:

- A. Provide all labor, products and equipment required to properly install an underslab vapor barrier under interior warehouse concrete floor slabs on grade. Refer to drawings for locations.
- Vapor barrier material shall be a multilayer polyolefin sheet material complying with ASTM E 1745, Class A, for a 10 mil thickness. Vapor barrier shall act as a vapor retarder having a water vapor permeance less than 0.0254 perms according to ASTM F 1249. Provide "Stego Wrap Class A Vapor Retarder" by Stego Industries, LLC or an approved equal.
- C. Provide all required accessory materials by the vapor barrier manufacturer, including seam tape and mastic. Accessory materials shall have a water vapor permeance of 0.3 perms or lower according to ASTM E 96.
- D. Installer shall proceed with application of the vapor barrier only after substrate construction and
- penetrating work have been completed and any unsatisfactory conditions have been corrected.

 As a general installation requirement, the installer shall comply with the vapor barrier manufacturer's written installation instructions and ASTM E 1643.
- F. Unroll the vapor barrier material with the longest dimension parallel with the direction of the concrete pour. Lap over footings or seal to foundation walls.
- G. Overlap joints 6 inches and seal with the seam tape.
- H. Seal all penetrations according to the manufacturer's written instructions
- . No penetration of vapor barrier material is allowed except for reinforcing steel and permanent utilities.
- J. Installer shall repair damaged areas by cutting patches of the vapor barrier material, overlapping the damaged area 6 inches and taping all four sides with specified tape.

VII. CONCRETE REINFORCEMENT:

- A. Reinforcing steel bars shall be Grade 60 (60 KSI ASTM A615) steel and tied with drawn steel wire.
- B. Welded Wire Fabric shall conform to ASTM A185.
- C. Placement of reinforcing steel shall be as shown on plans and per the "Manual of Standard Practice for Detailing Reinforced Concrete Structures", ACI 315 manual on placing steel reinforcing.
- D. Provide minimum lap splice of 48 bar diameters, but not less than 18 inches, for all reinforcing bars, unless noted otherwise. Stagger splices in adjacent bars at least 24 inches, except in strip footings,
- masonry tie beams (top of wall) and as noted otherwise.

 E. In wall footings, grade beams and bond beams, provide bent bars at corners and intersections of the same number and size as the straight bars.
- F. Provide for an allowance of 1% of the total reinforcing steel for the project to be fabricated and placed during progress of the work as may be directed by the Structural Engineer, in addition to the reinforcing steel indicated on the drawings. A credit for any unused quantity of this material at the end of the project shall be given to the Owner.

VIII. CAST IN PLACE CONCRETE:

- A. The concrete supplier shall provide mix designs of all concrete used in structural applications to Richard Adams Engineers.
- B. When pouring against earth, the embankment shall be wetted first and all pours on direct sun days will be misted at least twice following initial set. Isolation joints shall be provided at all columns and expansion felt at the perimeter of the slab.
- C. Finish on the surface of floor shall be machined troweled followed by hand finishing as required.
- D. Exterior walks and parking areas to be light brushed finished.
- E. Curing of concrete shall be in accordance with ACI 306.1 for cold-weather protection and with recommendations in ACI 305R for hot-weather protection during curing.

IX. TERMITE TREATMENT OF SOIL AND STRUCTURE:

- A. All buildings shall have pre-construction treatment protection against subterranean termites. A certification of compliance must be issued to the building department by a licensed pest control company before a certificate of occupancy will be issued. The certificate of compliance shall state: "The building has received a complete treatment for the prevention of subterranean termites. The treatment is in accordance with the rules and laws of Florida Department of Agriculture and Consumer Services." All of this work shall be in conformance with Florida Building Code Section 1816.
- A permanent sign that identifies the termite treatment provider and the need for re-inspection and treatment contract renewal shall be provided. The sign shall be posted near the water heater or electrical panel. The sign and the posting method shall be in accordance with Florida Building Code Section 105.1.1
- C. All condensate and roof downspouts shall discharge at least 1'-0" away from the structure sidewall, whether by underground piping, tail extensions, or splash blocks. Gutter with downspouts are required on all buildings with eaves of less than 6" horizontal projection (except gable end rakes or a roof above another roof). Irrigation/ sprinkler systems and risers for spray heads shall not be installed within one foot of the building sidewall. This placement of these water sources close to the building sidewall shall be in accordance with Florida Building Code Section 1503.6.
- D. The exterior wall covering shall terminate not closer than 6" from the final earth grade to allow for inspection for termite infestation. The exception to this is paint or decorative cementitious finishes less than 5/8" thick adhered directly to the masonry foundation sidewall. The termination of exterior wall coverings shall be in accordance with Florida Building Code Section 1403.7.
- E. If soil treatment is used for subterranean termite protection, the initial treatment shall be performed after ALL excavation, backfilling and compaction is complete. Any soil area that is disturbed after initial soil treatment shall be retreated with chemical treatment including spaces boxed or formed. This is required under Florida Building Code Section 1816.1.
- F. If soil treatment is used for subterranean termite protection, space in concrete floors boxed out or formed for subsequent installation of plumbing traps, drains, or any other purpose shall be created by using plastic or metal permanently placed forms of sufficient depth to eliminate any planned soil disturbance after initial chemical soil treatment. The placement and construction of the permanent forms shall be in accordance with Florida Building Code Section 1816.1.13.
- G. If soil treatment is used for subterranean termite protection, the chemical treated soil shall be protected against rainfall dilution by a 6 mil (min.) vapor barrier. If rainfall occurs prior to placement of the vapor barrier, chemical re-treatment is required.
- H. If soil treatment is used for subterranean termite protection, concrete overpour and mortar accumulation along the foundation perimeter shall be removed prior to exterior soil treatment
- If soil treatment is used for subterranean termite protection, chemical soil treatments shall also be applied under all exterior concrete or grade within one foot of primary structure sidewalls. Also, a vertical chemical treatment barrier shall be applied promptly after construction is completed, including initial landscaping and irrigation / sprinkler installation. Any soil disturbed after the chemical vertical barrier is applied shall be promptly retreated. This exterior chemical treatment shall be in accordance with Florida Building Code Section 1816.1.6.
- J. After all work is completed, loose wood and debris shall be completely removed from under the building and within one foot of the structure. All wood forms and supports shall be completely removed. This includes, but is not limited to: wooden grade stakes, forms, contraction spacers, tub trap boxes, plumbing supports, bracing, shoring, forms, or other cellulose-containing material place in any location where such materials are not clearly visible and readily removal prior to completion of the work
- K. At contractor's option, the building may be termite protected by "Centricon" (or equivalent) termite protection system in lieu of simple termite treated compacted subgrade.
 - 1. If a registered termiticide formulated and registered as a bait system is used for subterranean termite prevention, Sections 1816.1.1 through 1816.16 of the Florida Building Code do not apply; however, a signed contract assuring the installation, maintenance and monitoring of the baiting system for a minimum of 5 years from the issue of the certificate of occupancy shall be provided to the building official prior to the pouring of the slab, and the system must be installed prior to the final building approval. If the baiting system, directions for the use require a monitoring phase prior to installation of the pesticide active ingredient, the installation of the monitoring phase components shall be deemed to constitute installation of the system. This is required under Florida Building Code Section 1816.7.

BFRANKSTUDIO ARCHITECTURE | DESIGN | PLANNING

Hillsborough County Tax Collector Southshore Office

> 410 30th Street SE Ruskin, Florida 33570

OWNER Hillsborough County Tax Collector 601 E. Kennedy Blvd., 14th Floor Tampa, FL 33602 813.635.5200 www.hillstax.org

ARCHITECT BFrank Studio, LLC 4836 West Gandy Blvd tampa, florida 33611 813.769.9378 www.bfrankstudio.com

CONSTRUCTION MANAGER
Manhattan Construction Company
1715 N. Westshore Blvd, Suite 175
Tampa, FL 33607
813.675.1960
www.manhattanconstructiongroup.com

STRUCTURAL ENGINEER
Adams Engineers and Consultants,
Inc.
5507 E. Busch Blvd.
Tampa, FL 33617

813.985.4600

www.adams-engineers.com

MEPF ENGINEER
Hahn Engineering, Inc.
3060 South Dale Mabry Highway
Tampa, FL 33629
813.831.8599

Architect Business: B Frank Studio, FL Business License No: AA26003256
Beverly L. Frank Date: 02/28/2019
Architect License No: AR95616

LAURA M. MINTON, P.E. FLORIDA REG. # 67451

(NOT VALID WITHOUT SEAL)

Number Revision Description Date

Project No (2017-0009)
Date: DECEMBER 18, 2017

DRAWING TITLE:

B STRUCTURAL NOTES

R.A.E. JOB No. 17205

RICHARD ADAMS ENGINEERS

& CONSULTANTS, INC.

5507 E BUSCH BLVD TAMPA, FL 33617 PH: 813.985.4600 MAIL@ADAMS-ENGINEERS.COM FLORIDA CERTIFICATE OF AUTHORIZATION No. 7565 Sheet:

S001

CONFORMED SET

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X. TILT WALL PANELS:

- Shop drawings are required for work under this section. Shop drawings shall include the following;
 - Reinforcing layout
 - Embed layout
- Additional reinforcement required for panel erection
- Erection hardware placement and lift engineering
- Slab layout showing location of panel casting.
- B. The concrete supplier shall provide mix designs of all concrete used in the tilt wall panels.
- Chairs as required shall be all plastic. Steel chairs with plastic tips are not acceptable.
- D. No allowance was provided for the stresses created in the panels during erection. Contractor shall have panels investigated by a Professional Engineer licensed in the State of the Project to design panel lifting system. The results of this investigation shall be provided to this Engineer prior to the start of casting of the tilt wall panels.
- Rustification forming material shall be new and in unused condition unless form work is designed for
- Where "Rubber Bearing Pad" is specified at tilt wall spandrel bearing connections, these bearing pads shall meet or exceed the following criteria:
- Bearing pads shall be AASHTO Grade Neoprene Bearing Pads.
- These pads shall have a Durometer Hardness of 60 min.
- The bearing pad shall be 3/4" thick (unless noted otherwise).
- The bearing pads shall be black in color.
- G. Stack casting of tilt wall panels shall not be permitted with out prior written approval of the engineer. This approval shall limited to specific panels.
- H. All burrs, honeycomb and pockets shall be removed from panels after erection of same. This work shall begin immediately after the erection of the panels is complete.
- Panel Finishing: Unless otherwise indicated on plans, all panels shall receive a steel troweled finish on the interior surface. The exterior surface of the panels shall be cast against the floor slab, which shall have a steel troweled finish.
- Holes created by erection hardware and lifting devices shall be patched smooth. Prefabricated insert plugs are not acceptable
- K. Typical panel reinforcing shall continue uninterrupted through "knock out areas" in addition to the reinforcing required for the future opening.
- Holes left in the slab-on-grade due to panel braces shall be filled partially with a non shrink grout and topped with a minimum 2" layer of Dayton Superior (J-50) two part epoxy fill or approved equivalent. The epoxy top layer shall be ground smooth.

XI. FOUNDATIONS:

- Maximum soil bearing pressure used for design......2000 PSF
- Notify Engineer if footing excavation reveals unsuitable or unstable soils, or materials or conditions not anticipated in the original Report.
- Notify Engineer if footing excavation reveals unsuitable or unstable soils, or materials or conditions not anticipated in the area.
- D. Concrete placement shall occur immediately after footing excavation and placement of reinforcing
- steel. Any freestanding water shall be pumped out of footing excavation prior to concrete placement A qualified testing laboratory shall be retained to perform the following minimum in-place density tests: This suggested testing shall be used in the absence of a soil report or in the absence of a
- testing program suggested by a soil engineer as contained in a soil report. 1. One density test for each 2,500 square feet of compacted subgrade.
- One density test at each column.
- One density test per 50 feet of wall footing.
- F. In the absence of a soil report the soil compaction shall adhere to the following minimum
 - requirements for the Modified Proctor dry density test: 1. Slab-on-Ground: Shall be compacted to a minimum depth of one (1) foot below stripped grade. Any loose, soft or undesirable material shall be removed and replaced with structural fill is placed in loose lifts not exceeding 12" in depth:
 - Slabs 4" to 5" thick 93%
 - Slabs greater than 5" thick 95%
 - 2. Foundations: For foundations excavations that appear to have suitable bearing materials compaction shall be to one (1) foot below the bottom of the footing depth. When structural fill material is required sand fills shall be placed in loose lifts not exceeding 12" in depth.
 - a) Strip footings 24" wide or less, and pad footings with a footprint not
 - exceeding 16 sq. ft.: 93%
 - Strip footings greater than 24" wide and pad footings exceeding 16 sq. ft.: 97%
 - Back fill soils placed adjacent to footings, walls, or otherwise providing footing restraint shall be compacted in loose lifts not exceeding 6" in depth to 95%. To avoid damage the compaction shall be carefully done using either a light tired roller or vibratory plate compactor

XII. STEEL BAR JOISTS:

- A. The design, fabrication and erection of steel joists shall conform to the "Steel Joist Institute (SJI) "Standard Specifications and Load and Weight Table for Steel Joists" " (latest edition).
- Joists supplied under this section shall be fabricated by a recognized manufacturer, using cord or web sections fabricated from steel having a yield strength of at least 36 KSI but not exceeding 50
- Bridging per the applicable SJI Specification shall be used and shall be installed before construction loads are applied to the joists. The ends of all bridging lines terminating at walls or beams shall not be anchored until all the roof dead loads are applied.
- Ends of joists resting on steel supports shall be connected with the equivalent of two 1/8-inch fillet welds 2" long or with two 1/2" diameter bolts. Field welding shall not damage the joists.
- E. All bar joists shall be designed with a 150 pound Bend-Check (not additive) for both cords unless noted otherwise in drawings.
- F. All joists on column centerlines shall be secured by 1/2" inch diameter A325 bolts at the top chord bearing. The bottom chord shall be extended to the column.
- G. Joists shall bear 4" minimum on masonry and 2 1/2" minimum on steel U.N.O. Joists bearing on masonry shall bear on an embedded steel plate.
- H. Steel joists shall be primed painted with one coat of gray primer meeting the minimum requirements of SSPC-Paint 25 or Steel Structures Painting Council Specification 15-68T, Type I.
- Joist girders shall be proportioned such that they can be erected without bridging. The strutted ends of the bottom chord shall be restrained from lateral movement to brace the girder from overturning.

Bar joists and truss girders shall have manufacturer's standard beveled ends or sloped shoes if joist

slope exceeds 1/4 inches per 12 inches. K. Steel joists shall be designed to resist net uplift as shown on wind diagram shown in contract

XIII. STRUCTURAL STEEL:

- A. Fabrication and erection of structural steel shall be in accordance with AISC 360 "Specification for Structural Steel for Buildings" (latest edition).
- Structural steel team qualifications, unless otherwise approved, shall be:
 - 1. Fabricator Qualifications: A qualified fabricator who participates in the AISC Quality Certification Program and is designated an AISC Certified Plant, Category "Standard for Steel Building Structures" (STD) at time of bid.
- C. Structural steel shapes (used as beams and columns) shall conform to ASTM A 992 Grade 50 KSI unless otherwise noted on the contract drawings.
- D. Plates, channels, rods and angles shall conform to ASTM A36 unless otherwise noted on the contract drawings.
- Anchor rod (bolts) shall conform to ASTM F1554 Grade 36, washers shall conform to ASTM F436 Type 1, and nuts shall conform to ASTM A563. These rods shall be able to be welded.
- Steel pipe shall conform to ASTM A53 Grade B or ASTM A501
- Structural tubing shall conform to ASTM A500 Grade B (46 KSI minimum).
- All bolts (except anchor bolts) shall be high strength (HSB) shall conform to ASTM A325, 3/4" diameter unless noted otherwise. High strength bolts shall be used unless specifically noted on the drawings.
- All welding shall be performed by certified welders in accordance with AWS "Structural Welding Code-Steel" (D1.1-2010). The minimum electrode used shall be E70xx Low Hydrogen electrodes unless otherwise specified.
- Structural steel shall be primed painted with one coat of light gray primer meeting the minimum requirements of SSPC-Paint 25. All members to receive spray on fire proofing shall not be painted.
- All beams shall be fabricated and erected natural camber up
- Connections may be single shear plate (or double angle) framed beam connections per AISC unless noted otherwise. All bolts shall be 3/4" diameter A-325-N unless noted otherwise. Shop connections may be bolted or welded, with welded connections equal to bolted connections. All bolts shall be tightened snug tight with suitable nuts and hardened washers unless noted otherwise. Design connections for the larger of either the shear value shown on the drawings or 75% of the maximum shear listed in the tables for "Allowable uniform load in Kips for beams laterally supported" at the bottom of each page in the "Properties And Reaction Values" Part 2 of the latest edition of the AISC "Manual Of Steel Construction".
- M. Before erection proceeds, and with the steel erector present, verify elevations of concrete and masonry bearing surfaces and locations of anchorages for compliance with requirements. Do not proceed with erection until unsatisfactory conditions have been corrected.
- N. Do not enlarge unfair holes in members by burning or by using drift pins. Ream holes that must be enlarged to admit bolts.
- O. Splicing of structural steel where not detailed is not permitted with out prior written approval of the structural engineer.
- P. Structural openings, supports, anchors, etc. around or affected by mechanical, electrical and plumbing equipment shall be verified with the equipment purchased before proceeding with structural work affected.

XIV. METAL ROOF DECK:

- A. Metal roof deck shall be 1-1/2" thick, 22 Ga. Type B (as identified by the Steel Deck Institute) painted gray steel deck conforming to ASTM A611 with minimum yield stress of 80 ksi. Deck finish shall be shop primed with baked-on, lead- and chromate-free rust-inhibitive primer complying with performance requirements of SSPC-Paint 25.
- The deck shall be placed on the supporting framework with a minimum end lap of two inches centered over supports. The deck shall be attached to the supports, and the side lap of adjacent units in the pattern shown on the contract drawings.
- All roof deck openings 12" diameter or larger are to have support angles per typical deck opening detail, including openings for roof sump pans.
- Roof deck shall be laid out such that decking shall span three spans without interruption wherever possible
- E. Deck and supporting members damaged by excess welding heat shall be repaired or replaced as determined by Engineer.
- Puddle welds shall be at least 5/8" in effective diameter or an elongated weld having an equal perimeter. Fillet and seam welds when used shall be a minimum of 1 1/2" long. Weld metal shall penetrate all layers of deck material at end laps and side joints and have good fusion to the supporting members.
- The General Contractor shall obtain the services of a certified steel inspector to review all steel connections. This includes bolted connections, welded connections, and metal deck attachment. The certified inspection reports shall be provided to Richard Adams Engineers and the Local Jurisdiction Having Authority as required.

XV. LIGHT GAUGE STEEL FRAMING:

- A. The light gauge steel framing members specified are based on structural properties of Dietrich Industries. Members may be supplied by other manufacturers provided the members meet or exceed the structural properties as specified in Dietrich Industries' catalog.
- Structural properties and capacities of steel framing components are as computed in accordance with the latest edition of the A.I.S.I. Cold-Formed Design Specification.
- All stud and joist members 16 gage (54 mils) and heavier shall be formed from steel corresponding to a type listed in the A.I.S.I Specification for the Design of Cold-Formed Steel Structural Members, with the minimum yield strength of 50 ksi.
- All 18 gage (43 mils) and lighter members, and all track, bridging, and accessory items shall be formed from steel meeting the criteria as listed above, with the minimum yield strength of 33 ksi unless specifically noted otherwise.
- All welded connections are to be performed in accordance with the latest version of AWS D1.3 Specifications for Welding Sheet Steel in Structures. Consult AWS D19.0 Welding Zinc Coated Steel and ANSI standard Z49.1 for information regarding safe welding procedures.
 - 1. Suggested weld metal and process for shop welding are: 60 ksi weld metal strength
 - 2. Suggested methods for field welding: 1/8" (unless noted otherwise) E60XX (minimum) electrode - SMAW; or "gasless" MIG.
- Minimum weld throat thickness (t) must match or exceed the base steel thickness of the thinnest connected part unless noted otherwise.
- F. The light gauge clips and hangers noted in these drawings are manufactured by Dietrich Metal Framing. Contractor may submit other manufacturer's clips for approval provided the alternate clips meet or exceed the structural properties of the specified clips.
- G. Unless noted otherwise, refer to literature published by Hilti Fastening Systems, Inc. for expansion bolt, or powder driven fastener information; Buildex, Inc. for TEKS screw data. Alternate manufacturer's fasteners of comparable specifications and load capacities are acceptable.
- All framing components shall be cut squarely for attachment to perpendicular members or as required for an angular fit against abutting members. Members shall be held positively in place until
- All field cutting of studs must be done by sawing or shearing. Torch cutting of cold-formed members is unacceptable.

fasteners. Concrete anchors shall be installed after full compressive strength has been achieved.

- Track shall be securely anchored to the supporting structures as shown using the specified
- K. Studs shall be plumbed, aligned and securely attached to the flange or webs of both upper and lower tracks. Wall studs that contain bridging in the stud punchout, the framing fabricator is to ensure punchout alignment when assembling framing and field cutting studs to length.
- No splices in studs, joists, or other load carrying members may be made without prior engineering review and specific details for any such splice(s).
- Wall study bridging shall be attached in a manner to prevent stud rotation.
- Temporary bracing shall be provided until erection is completed.
- O. Details of wall finishes are for arrangement and location. For specific requirements, methods, materials, and execution standards, refer to technical data from product manufacturer. In the event of conflict, manufacturer's instructions shall dictate.

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ARCHITECTURE | DESIGN | PLANNING

Hillsborough County Tax Collector Southshore Office

> 410 30th Street SE Ruskin, Florida 33570

Hillsborough County Tax Collector 601 E. Kennedy Blvd., 14th Floor Tampa, FL 33602 813.635.5200 www.hillstax.org

ARCHITECT BFrank Studio, LLC 4836 West Gandy Blvd tampa, florida 33611 813.769.9378 www.bfrankstudio.com

CONSTRUCTION MANAGER Manhattan Construction Company 1715 N. Westshore Blvd, Suite 175 Tampa, FL 33607 813.675.1960 www.manhattanconstructiongroup.com

STRUCTURAL ENGINEER Adams Engineers and Consultants 5507 E. Busch Blvd. Tampa, FL 33617 813.985.4600

MEPF ENGINEER Hahn Engineering, Inc. 3060 South Dale Mabry Highway Tampa, FL 33629 813.831.8599

www.adams-engineers.com

B Frank Studio, LLC Architect Business: FL Business License No: AA26003256 Beverly L. Frank Date: 02/28/2019 Architect License No: AR95616

> LAURA M. MINTON, P.E. FLORIDA REG. # 67451 (NOT VALID WITHOUT SEAL)

Number Revision Description Date

Project No **DECEMBER 18, 2017** Date:

Sheet:

DRAWING TITLE

STRUCTURAL NOTES

(2017-0009)

SOO2

CONFORMED SET

R.A.E. JOB No. 17205 RICHARD ADAMS ENGINEERS & CONSULTANTS, INC. 5507 E BUSCH BLVD TAMPA, FL 33617 PH: 813.985.4600 MAIL@ADAMS-ENGINEERS.COM FLORIDA CERTIFICATE OF AUTHORIZATION No. 7565

XVI. DELEGATED (SPECIALTY) ENGINEERING REQUIREMENTS: A. GENERAL REQUIREMENTS:

- 1. See "Submittal Requirements Table" in these drawings for structural elements / products which require submittals.
- 2. Definition of Delegated (Specialty) Engineer: A Professional Engineer, who is licensed in the same state as the Project, not the Structural Engineer of Record, who specializes in and who undertakes the design of structural components prepared for this Project.
- 3. Submittals for custom designed, manufactured or fabricated load-carrying items and custom fabricated items that are required by codes or standards to resist forces and stresses, including their connections, anchorages, and attachments require a Delegated Engineer.
- 4. For each category of submittal requiring input from a Delegated Engineer, the Contractor shall attach to the first submittal a signed and sealed letter from the responsible Delegated Engineer stating "I certify that the design and drafting of the shop drawings which are signed and sealed by me were prepared under my direct supervision and control and to the best of my knowledge the shop drawings comply with the applicable minimum building codes and contract drawings."
- 5. Review by the Structural Engineer of Record of submittals is LIMITED TO the following:
- The specified structural submittals have been furnished.
- The structural submittals have been signed and sealed by the Delegated Engineer.
- The Delegated Engineer has understood the design intent and has used the specified structural criteria. No detailed check of calculations will be made.
- The configuration set forth in the structural submittals is consistent with the contract
- documents. No detailed check of dimensions or quantities will be made. 6. SUBMITTALS NOT MEETING THE ABOVE CRITERIA WILL NOT BE REVIEWED.

XVII. POST-INSTALLED ANCHORS:

The below Products are the design basis for this project. Substitution requests for products other than those listed below may be submitted by the contractor to the Engineer-of-Record (EOR) for review. Substitutions will only be considered for products having a code Report recognizing the product for the appropriate application and project building code. Substitution submittals shall demonstrate that the substituted product is capable of achieving the equivalent performance values of the design basis product.

- A. Adhesive Anchors (Epoxy):
 - Into Concrete: Adhesive for rebar and anchors shall have been tested in accordance with ACI 355.4 and ICC-ES AC308 for cracked concrete and seismic applications. Pre-approved products include:
 - HILTI HIT-200 (ICC-ES ESR-3187)
 - SIMPSON STRONG-TIE "SET-XP" (ICC-ES ESR-2508)
 - SIMPSON STRONG-TIE "AT-XP" (IAPMO-UES ER-0268)
 - 2. Into Solid Grouted CMU: Adhesive for rebar and anchors shall have been tested in accordance ICC-ES AC58. Pre-approved products include:
 - HILTI HIT-HY 70 Masonry Adhesive Anchoring System (ICC-ESR-3342)
 - SIMPSON STRONG-TIE "SET-XP" (IAPMO-UES ER-265)
 - SIMPSON STRONG-TIE "AT-XP" (IAPMO-UES ER-0281)
 - 3. Into Hollow/ Multi-Wythe Masonry: Adhesive for rebar and anchors shall have been tested in accordance with ICC-ES AC58. The appropriate screen tube shall be used as recommended by the adhesive manufacturer. Pre-approved products include:
 - HILTI HIT-HY 70 Masonry Adhesive Anchoring System (ICC-ESR-3342)
 - b) SIMPSON STRONG-TIE "SET" (ICC-ES ESR-1772)
 - 4. Anchoring System shall utilize traditional preparation of the anchor hole (Blowing and brushing) per the manufactures requirements. Other methods (i.e. no cleaning with HIT-Z rods or hollow drill bits/vacuum for the Hilti HIT-200 system) may not be used without EOR 5. Anchoring adhesive shall be a two-part component 100% solid epoxy based system supplied
 - through a static-mixing nozzle supplied by the manufacturer. This requirement shall be met regardless of which epoxy product or manufacturer that is used on this project.
 - 6. The threaded rods to be used in combination with Epoxy system shall be fabricated from steel meeting or exceeding the properties of ASTM A36.
- B. Mechanical Anchors (Expansion/Screw Anchors)
 - 1. Into Concrete: Anchors shall have been tested in accordance with ACI 355.2 and ICC-ES AC193 for cracked concrete and seismic applications. Adhesive anchors shall be installed by a certified adhesive anchor installer Where designated on the contract documents. Pre-approved products include:
 - a) Screw Anchor- HILTI "KWIK HUS-EZ" (ICC-ES ESR 3027)
 - Expansion Anchor- HILTI "KWIK BOLT TZ" (ICC-ES ESR 1917)
 - Screw Anchor- SIMPSON STRONG-TIE "TITEN-HD" (ICC-ES ESR 2713)
 - Expansion Anchor- SIMPSON STRONG-TIE "STRONG-BOLT 2" (ICC ES
 - ESR-3037) OR "WEDGE-ALL" (ICC-ES ESR-1396)
 - 2. Into Solid Grouted CMU: Anchors shall have been tested in accordance with ICC-ES AC01 or ICC-ES AC106. Pre-approved products include:

 - a) Screw Anchor- HILTI KWIK HUS-EZ (AC 106-ESR Pending)
 - Expansion Anchor- HILTI "KWIK BOLT 3" (ICC-ES ESR 1385)
 - Screw Anchor- SIMPSON STRONG-TIE "TITEN-HD" (ICC-ES ESR 1056)
 - Expansion Anchor- SIMPSON STRONG-TIE "STRONG-BOLT 2" (IAPMO UES ER-240)

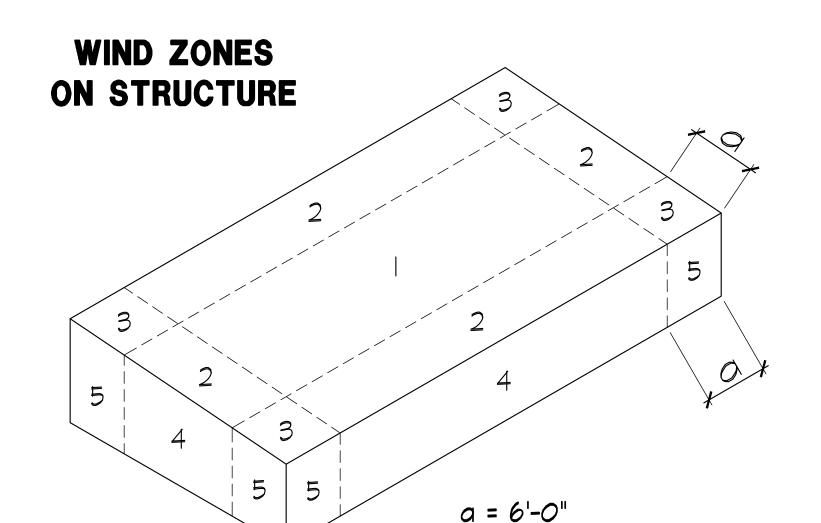
	ADDITIONAL	۲.	WAGE OUT
	-ADDITIONAL		
	-ARCHITECT		-KNOCK OUT BLOCK
	-BOTTOM OF	LG	-LONG
			-LONG LEG HORIZONTAL -LONG LEG VERTICAL
			-LOW POINT
	-EXPANSION JOINT MATERIAL -CENTER LINE		-PRE ENGINEERED METAL BUILDING -POURED IN PLACE
	-CLEAR		-MOMENT CONNECTION
	-CONCRETE MASONRY UNIT		
			-MINIMUM -NOT IN CONTRACT
	-COLUMN	N.I.C. N.S.	-NEAR SIDE
	-CONCRETE		
	-CONTINUOUS	N.T.S.	-NOT TO SCALE
	-DOUBLE		-ON CENTER
	-DIAMETER		-ON CENTER
	-EXISTING		-OPPOSITE HAND
	-EXISTING		-POWDER DRIVEN FASTENER
	-EACH		
	-EACH END		-PLATE
	-EACH FACE		-REINFORCEMENT
	-EACH WAY		-ROOF DRAIN
	-EXPANSION JOINT		-STEEL BAR JOIST
	-ELEVATION		-SAMED CONTRACTION JOINT
	-ELEVATOR		-SLAB ON GRADE
E.O.S.			-TOP AND BOTTOM
EQ	-EQUAL		-TURN DOWN SLAB
	-EXPANSION		-THICKENED EDGE SLAB
	-FLOOR DRAIN		-THICKENED EDGE MONOLITHIC SLAB
	-FINISHED FLOOR		-THICK
FND	-FOUNDATION		-TOP OF CONCRETE
	-FAR SIDE		-TOP OF FOOTING
FTG	-FOOTING		-TOP OF STEEL
F.V.			-TOP OF TIE BEAM
	-GAUGE		-TOP OF WALL
GALV	-GALVANIZED	T.O.M.	-TOP OF MASONRY
HD	-HEADED	TYP	-TYPICAL
H.D.G.	-HOT DIPPED GALVANIZED	U.N.O.	-UNLESS NOTED OTHERWISE
	-HORIZONTAL	V	-SHEAR VALUE
H.P.	-HIGH POINT	VERT	-VERTICAL
H.R.	-HAND RAIL	V.I.F.	-VERIFY IN FIELD
H.S.S.	-HOLLOW STEEL SECTION	M/	-MITH
	-HOLLON STEEL SECTION		
H.S.	-HIGH STRENGTH	W/O	-WITHOUT
		W/O WP	

RAE ABBREVIATION KEY

NOTE:
THIS KEY PERTAINS TO THIS JOB ONLY AND MAY DIFFER FROM OTHER PROJECTS
PRODUCED BY THIS OFFICE AND THOSE USED BY OTHER FIRMS AND DISCIPLINES

DESIGN LOAD SCHEDULE						
ALL LOADS SHOWN IN POUNDS PER SQUARE FOOT						
TYPICAL ROOF						
6.0						
2.0						
2.0						
2.5						
1.5						
2.0						
2.0						
2.0						
20.0						
20.0						
40.0						
	6.0 2.0 2.5 1.5 2.0 2.0 2.0					

Minimum Required Submittals										
	Epoxy and Mechanical Anchors	Misc. Conc Products	Concrete Mix Design	Light Gauge Members and Connections	Metal Deck	Steel Bar Joists	Structural Steel & Connections	Tilt Wall Bracing	Tilt Wall Elevations (Embe Layout)	Rebar
Product Literature	•	•	•	•						
Shop Drawings					•	•	•		•	•
Shop Drawings (Signed and Sealed)								•		
Calculations (Signed and Sealed)										
Shop Drawings & Calculations (Signed and Sealed)										



ALLOWABLE STRESS DESIGN (ASD)

POSITIVE WIND PRESSURES ON GLAZING & WALL COMPONENTS ASD									
LOCATION ON BUILDING	POS. PRESSURE < 10 SQ. FT.	POS. PRESSURE < 20 SQ. FT.	POS. PRESSURE < 50 SQ. FT.	POS. PRESSURE < 100 SQ. FT.					
FIELD AREA ZONE 4	25.16 PSF	24.12 PSF	22.65 PSF	21.6 PSF					
CORNER AREA ZONE 5	25.16 PSF	24.12 PSF	22.65 PSF	21.6 PSF					

NEGATIVE WIND PRESSURES ON GLAZING & WALL COMPONENTS ASD								
LOCATION ON BUILDING	NEG. PRESSURE < 10 SQ. FT.	NEG. PRESSURE < 20 SQ. FT.	NEG. PRESSURE < 50 SQ. FT.	NEG. PRESSURE < 100 SQ. FT.				
FIELD AREA ZONE 4	27.26 PSF	26.21 PSF	24.74 PSF	23.7 PSF				
CORNER AREA ZONE 5	33.55 PSF	31.46 PSF	28.52 PSF	26.21 PSF				

ROOF UPLIFT WIND PRESSURES ASD									
LOCATION ON ROOF	NEG. PRESSURE < 10 SQ. FT.	NEG. PRESSURE < 20 SQ. FT.	NEG. PRESSURE < 50 SQ. FT.	NEG. PRESSURE < 100 SQ. FT.	NET UPLIFT PRESSURE ON BAR JOISTS				
FIELD AREA ZONE 1	27.5 PSF	26.8 PSF	25.87 PSF	25.16 PSF	21.56 PSF				
EDGE AREA ZONE 2	46.14 PSF	41.24 PSF	34.96 PSF	29.83 PSF	26.23 PSF				
CORNER AREA ZONE 3	69.44 PSF	57.56 PSF	41.95 PSF	29.83 PSF	26.23 PSF				

STRENGTH DESIGN (LRFD)

POSITIVE WIND PRESSURES ON GLAZING & WALL COMPONENTS POS. PRESSURE POS. PRE								
LOCATION ON BUILDING	< 10 SQ. FT.	< 20 SQ. FT.	< 50 SQ. FT.	< 100 SQ. FT.				
FIELD AREA ZONE 4	41.94 PSF	40.2 PSF	37.75 PSF	36 PSF				
CORNER AREA ZONE 5	41.94 PSF	40.2 PSF	37.75 PSF	36 PSF				

NEGATIVE WIND PRESSURES ON GLAZING & WALL COMPONENTS								
LOCATION ON BUILDING	NEG. PRESSURE < 10 SQ. FT.	NEG. PRESSURE < 20 SQ. FT.	NEG. PRESSURE < 50 SQ. FT.	NEG. PRESSURE < 100 SQ. FT.				
FIELD AREA ZONE 4	45.44 PSF	43.69 PSF	41.24 PSF	39.5 PSF				
CORNER AREA ZONE 5	55.92 PSF	52.43 PSF	47.54 PSF	43.69 PSF				

ROOF UPLIFT WIND PRESSURES								
LOCATION ON ROOF	NEG. PRESSURE < 10 SQ. FT.	NEG. PRESSURE < 20 SQ. FT.	NEG. PRESSURE < 50 SQ. FT.	NEG. PRESSURE < 100 SQ. FT.				
FIELD AREA ZONE 1	45.83 PSF	44.66 PSF	43.11 PSF	41.94 PSF				
EDGE AREA ZONE 2	76.9 PSF	68.74 PSF	58.26 PSF	49.71 PSF				
CORNER AREA ZONE 3	115.73 PSF	95.93 PSF	69.91 PSF	49.71 PSF				

WIND LOAD BASED ON USING ASCE 7-10 "MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES" METHODS OF CALCULATIONS FOR WIND PRESSURES BASED ON THE FOLLOWING FACTORS:

- 1. WIND SPEED OF 145 MPH. ASD VELOCITY OF 112 MPH.
- 2. BUILDING RISK CATEGORY II PER ASCE-7.
- 3. "ENCLOSED" BUILDING CLASSIFICATION (Gcpi = +/- 0.18). 4. BUILDING EXPOSURE CATEGORY OF C PER ASCE-7.
- 5. MEAN ROOF HEIGHT OF LESS THAN 15 FEET.
- 6. ROOF ANGLE OF LESS THAN 7 DEGREES.
- 7. ALL GLAZING IS ASSUMED PROTECTED IN ACCORDANCE WITH SECTION 1609.1.2 OF THE FLORIDA BUILDING CODE.
- 8. ASD PRESSURE TABLES HAVE BEEN FACTORED BY 0.6 PER ASCE SECTION 2.4.1



Hillsborough County Tax Collector Southshore Office

> 410 30th Street SE Ruskin, Florida 33570

Hillsborough County Tax Collector 601 E. Kennedy Blvd., 14th Floor Tampa, FL 33602 813.635.5200 www.hillstax.org

ARCHITECT BFrank Studio, LLC 4836 West Gandy Blvd tampa, florida 33611 813.769.9378 www.bfrankstudio.com

CONSTRUCTION MANAGER Manhattan Construction Company 1715 N. Westshore Blvd, Suite 175 Tampa, FL 33607 813.675.1960 www.manhattanconstructiongroup.com

Adams Engineers and Consultants,

5507 E. Busch Blvd. Tampa, FL 33617 813.985.4600 www.adams-engineers.com MEPF ENGINEER

STRUCTURAL ENGINEER

Hahn Engineering, Inc. 3060 South Dale Mabry Highway Tampa, FL 33629 813.831.8599

Architect Business: B Frank Studio, LLC FL Business License No: AA26003256 Beverly L. Frank Date: 02/28/2019 Architect License No: AR95616

> LAURA M. MINTON, P.E. FLORIDA REG. # 67451 (NOT VALID WITHOUT SEAL)

Number Revision Description Date

(2017-0009) Project No **DECEMBER 18, 2017** Date:

DRAWING TITLE:

Sheet:

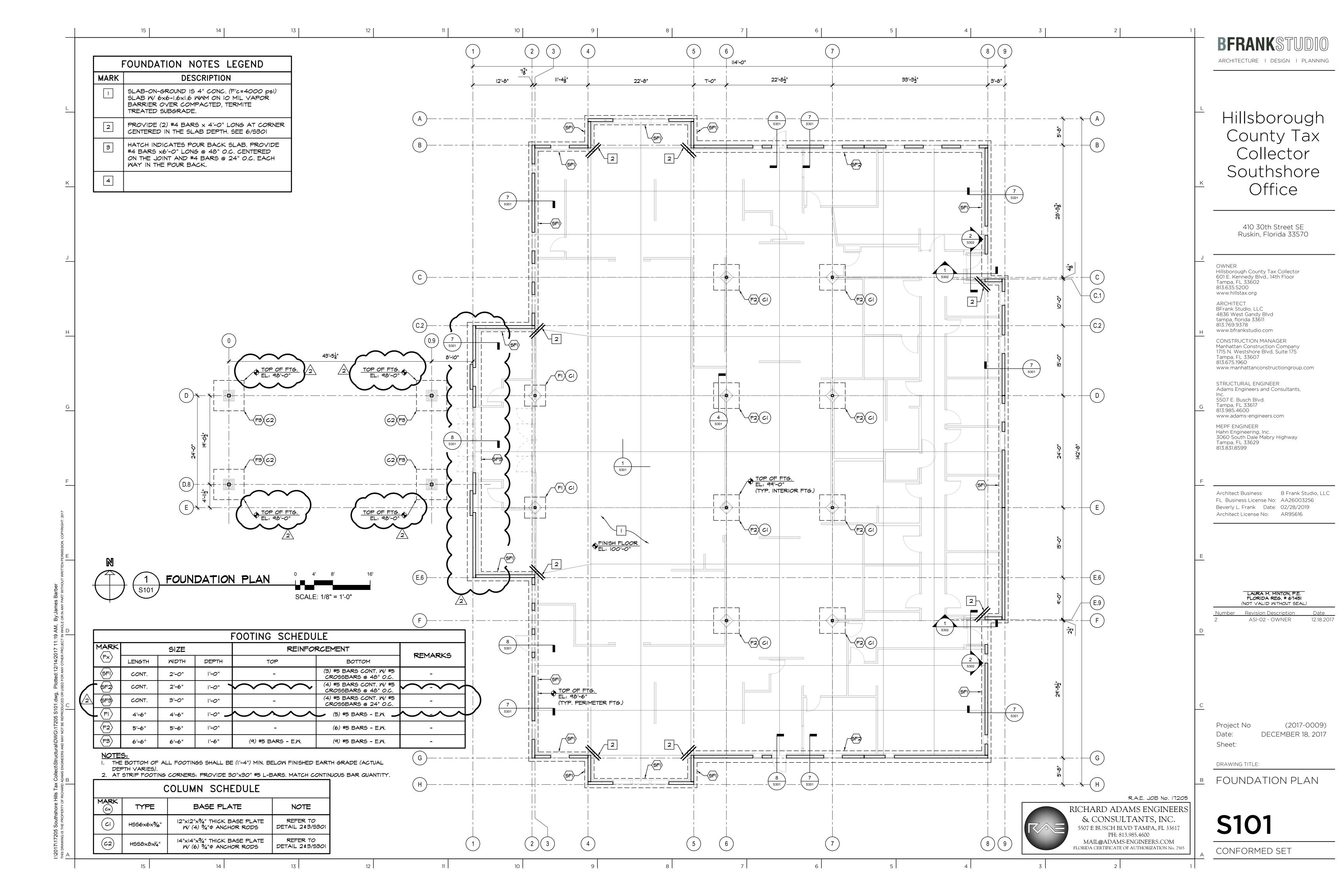
STRUCTURAL NOTES

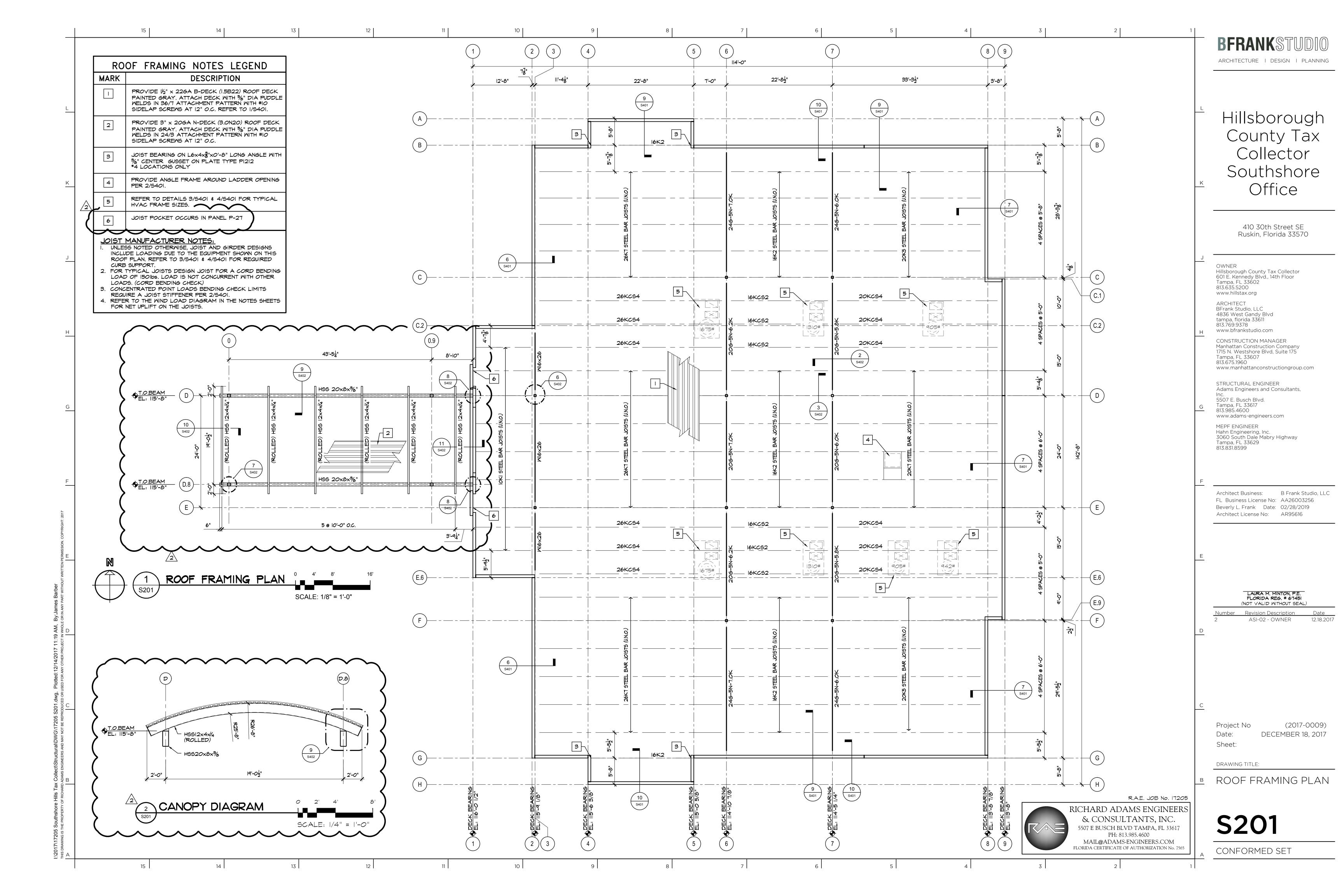
RICHARD ADAMS ENGINEERS & CONSULTANTS, INC. 5507 E BUSCH BLVD TAMPA, FL 33617 PH: 813.985.4600

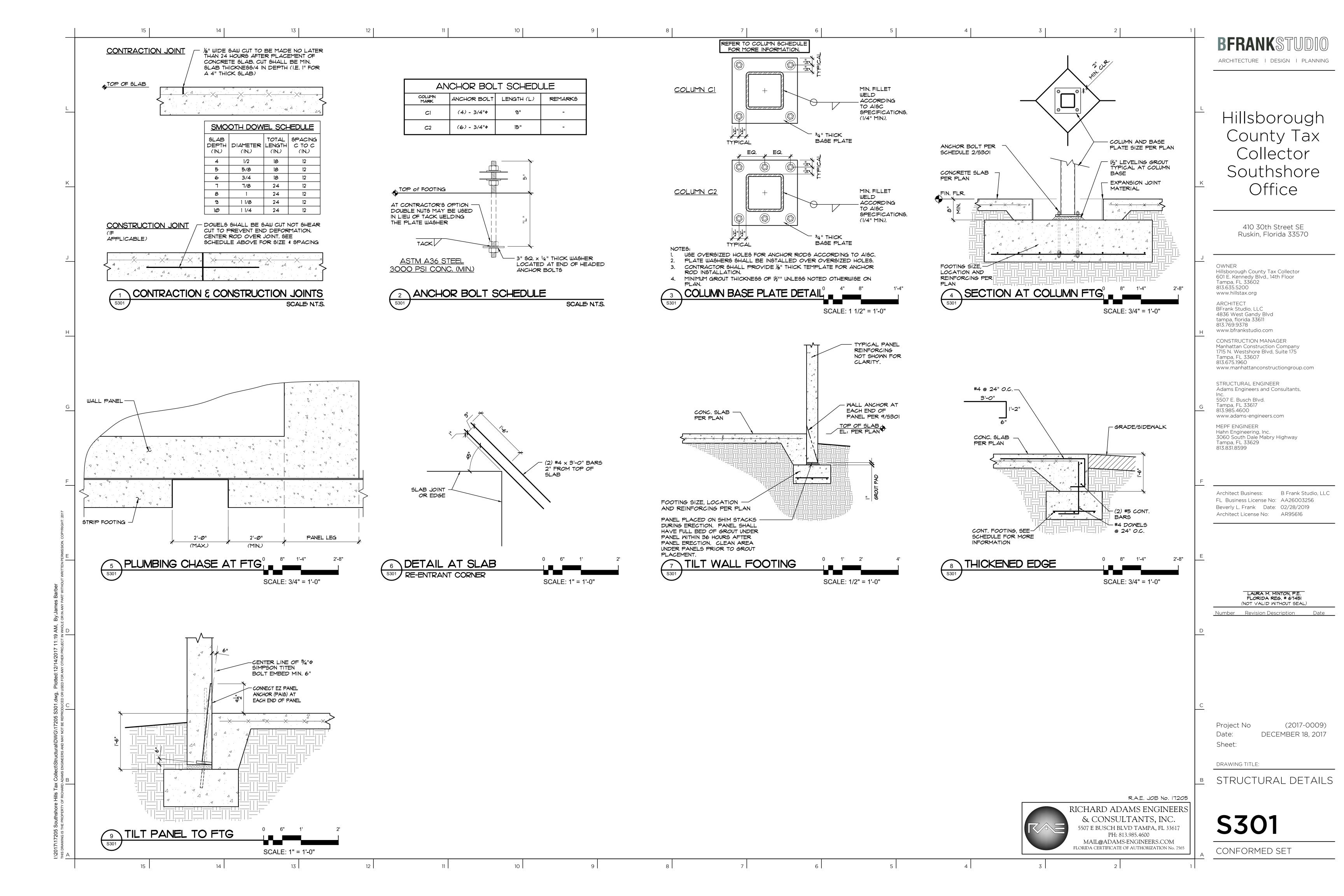
R.A.E. JOB No. 17205

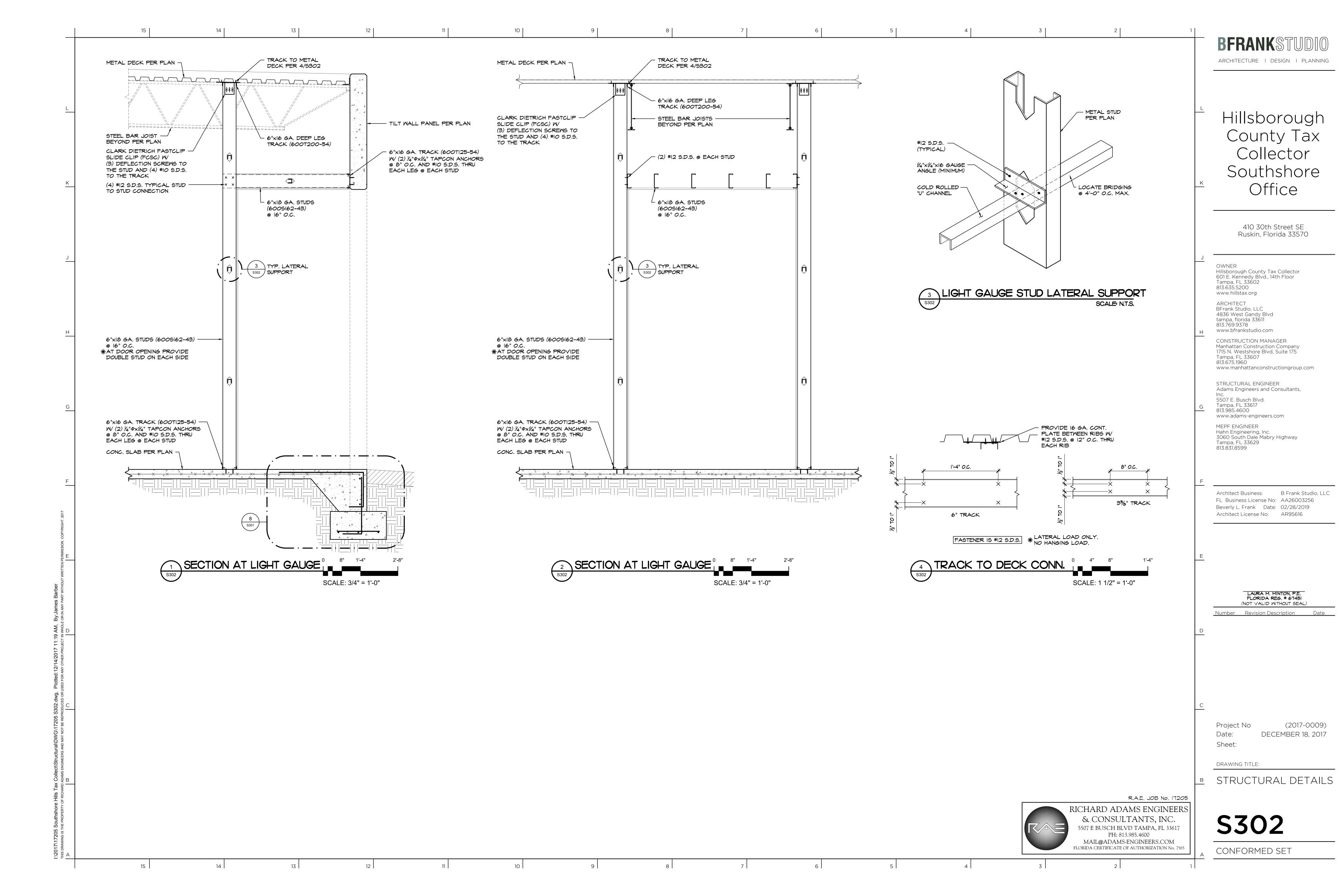
MAIL@ADAMS-ENGINEERS.COM FLORIDA CERTIFICATE OF AUTHORIZATION No. 7565

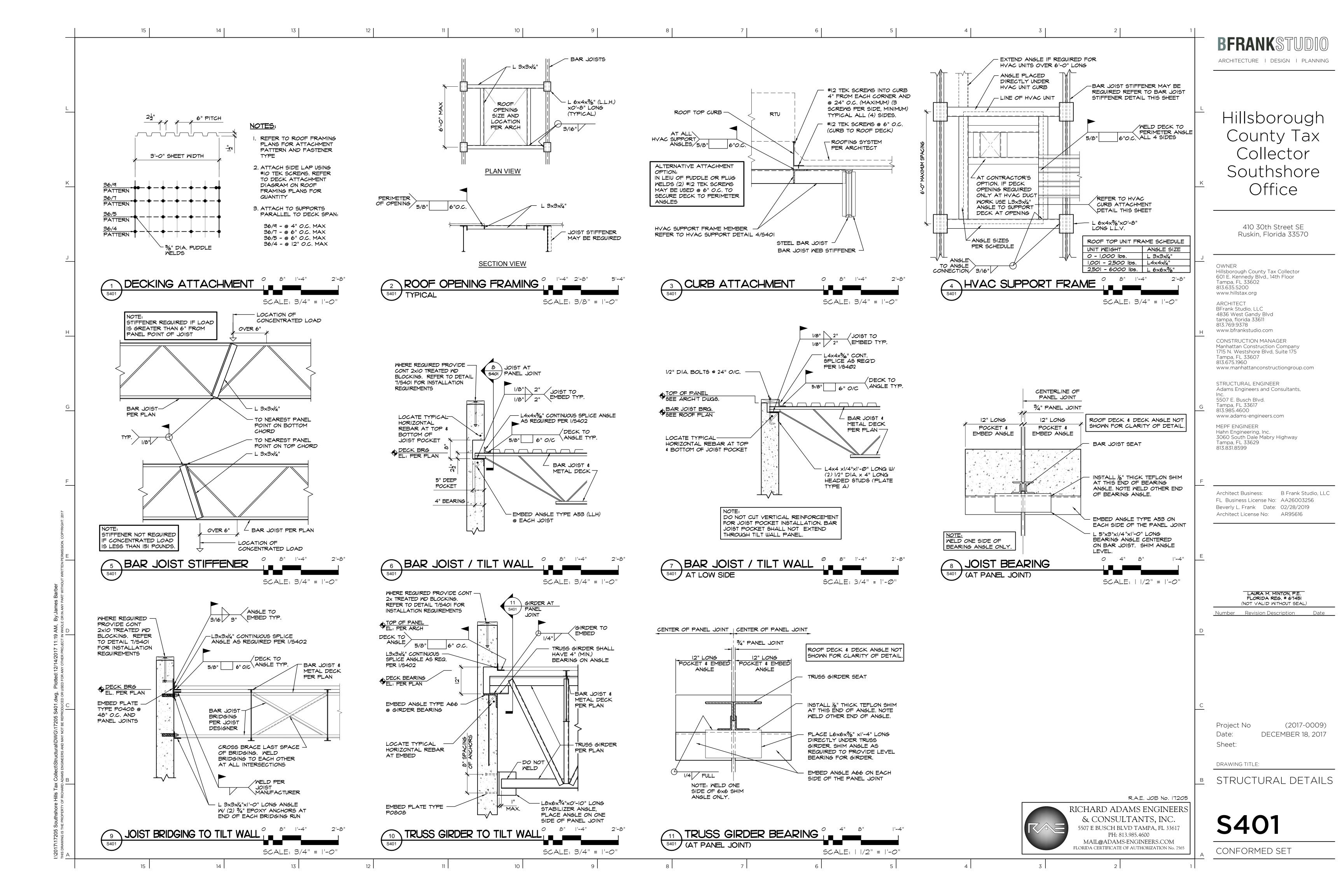
CONFORMED SET

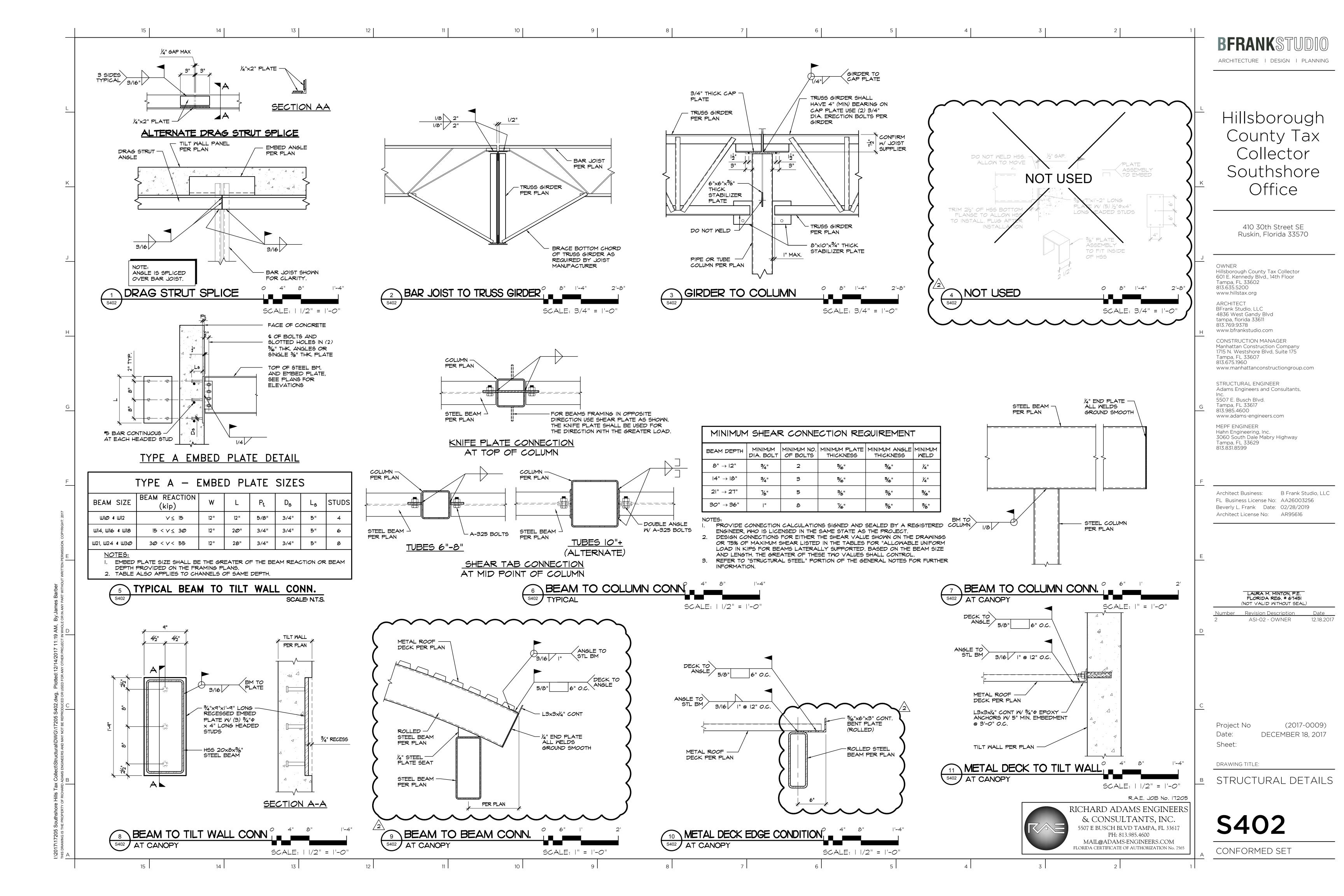


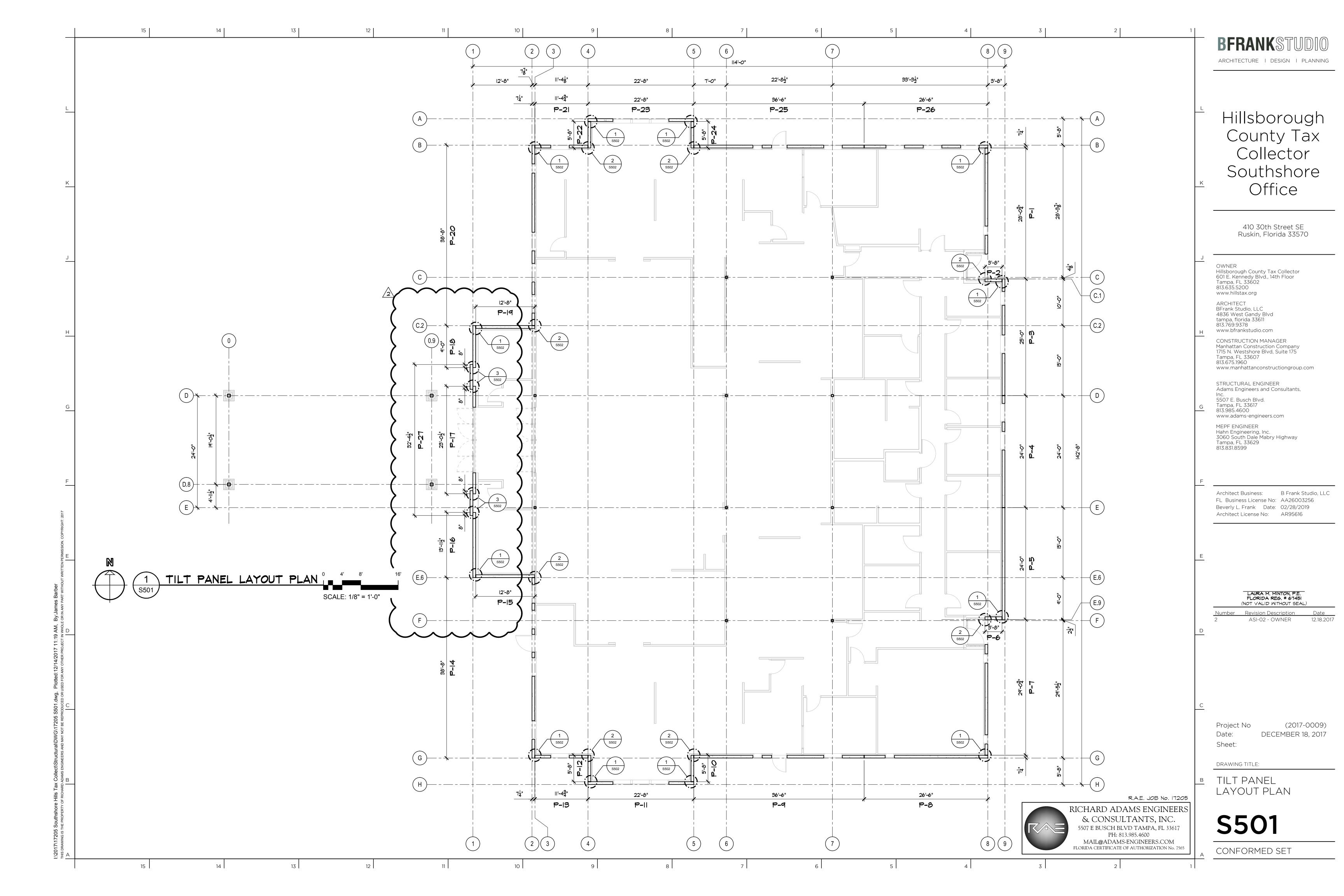


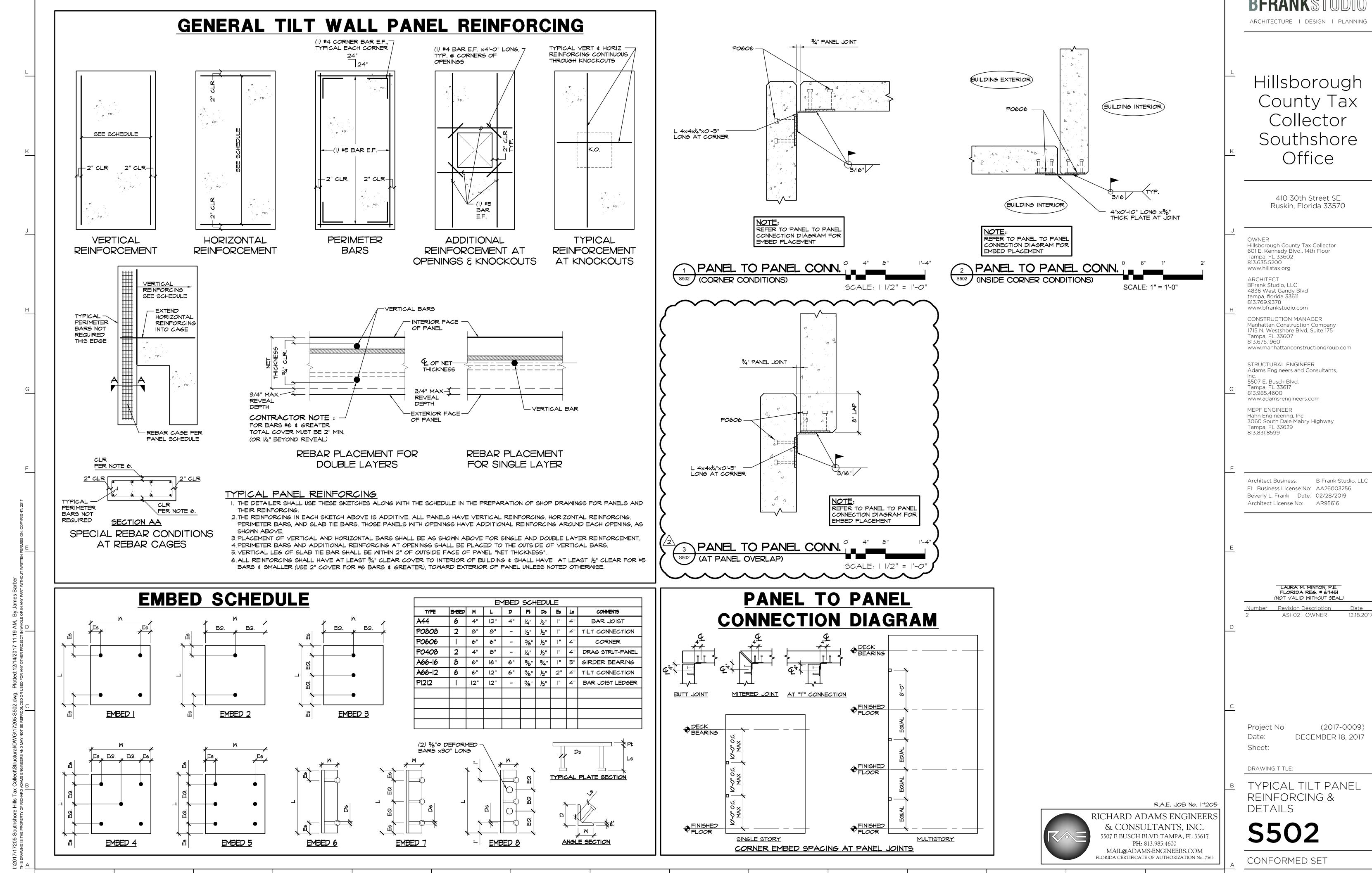












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